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**MIRAGE PANELS, PBU-PANELS, U-PANELS,  
MEGA-RIB PANELS, MIRAGE II PANELS,  
MULTI-RIB PANELS, MULTI-RIB PB PANELS**

### CSI Section:

07 41 13 – Metal Roof Panels

07 42 13 – Metal Wall Panels

07 42 13.13 – Formed Metal Wall Panels

## 1.0 RECOGNITION

Mirage Panels, PBU-Panels, U-Panels, Mega-Rib Panels, Mirage II Panels, Multi-Rib Panels, and Multi-Rib PB Panels described in this report have been evaluated for use as metal roof and wall panels. The structural, weather resistance, and fire performance properties of the Mirage Panels, PBU-Panels, U-Panels, and Mega-Rib Panels have been evaluated for compliance with the following codes:

- 2018, 2015, 2012, and 2009 International Building Code® (IBC)
- 2018, 2015, 2012, and 2009 International Residential Code® (IRC)

## 2.0 LIMITATIONS

Use of the Mirage Panels, PBU-Panels, U-Panels, Mega-Rib Panels, Mirage II Panels, Multi-Rib Panels, and Multi-Rib PB Panels described in this report is subject to the following limitations:

**2.1** Mirage Panels, PBU-Panels, U-Panels, Mega-Rib Panels, Mirage II Panels, Multi-Rib Panels, and Multi-Rib PB Panels shall be installed and used in accordance with this report, applicable code requirements, and the manufacturer's published installation guidelines. Where conflicts occur, the more restrictive requirements shall govern.

**2.2** Mirage Panels, PBU-Panels, U-Panels, Mega-Rib Panels, Mirage II Panels, Multi-Rib Panels, and Multi-Rib PB Panels roof slopes shall comply with IBC Section 1507.4.2 or IRC Section R905.10.2.

**2.3** Details and calculations demonstrating compliance with this report shall be submitted to the building official for approval. When required by the local jurisdiction, the structural calculations and related documents shall be prepared by a registered design professional.

**2.4** Design of panel penetrations and other panel discontinuities shall be the responsibility of the design professional using rational engineering mechanics or in accordance with the manufacturer's installation instructions as approved by the building official.

**2.5** Use of the Mirage and Mirage II panels as lateral load resisting elements in horizontal or vertical diaphragms is outside the scope of this report.

**2.6** Where PBU-Panels, U-Panels, Mega-Rib Panels, Multi-Rib Panels, and Multi-Rib PB Panels wall panels are used as vertical diaphragm shear resistance in walls (shear wall) of light-frame construction, for seismic design, the walls shall be classified as a "bearing wall system" or "building frame system" with "light-framed walls with shear panels of all other materials" subject to the conditions of this classification as defined in ASCE/SEI 7 Section 12.2.

**2.7** When steel panels are used as the stressed skin shear carrying element of a horizontal or sloped diaphragm as defined in Section 202 of the IBC, the diaphragm length and width shall be limited by one of the following: engineering mechanics; applied loads; shear capacity of the diaphragm; diaphragm shear deflection limited by the requirements of ASCE/SEI 7 in Section 12.8.6 entitled, "Story Drift Determination"; or Section 12.12 entitled, "Drift and Deformation". Shear deflection shall be based on the shear stiffness for the steel deck diaphragm and equations of mechanics. Common shear deflection equations as shown in Table 19 of this report may be used.

## 2.8 Product Performance

**2.8.1 Structural:** The tables provided in this report specify the gross and effective section properties, inward (positive) uniform allowable loads, allowable reactions at supports, outward (negative) uniform allowable loads, allowable diaphragm shear strengths,  $q$  (plf), and shear stiffnesses,  $G$  (kips/in) for each of the panels described in Section 4.0 of this report.

**2.8.2 Roof Classification:** Roof assemblies complying with the requirements of IBC Section 1505.2, Exception 2, or IRC Section R902.1, Exception 2, are considered Class A roof assemblies. For other conditions, roof assemblies shall be listed as Class A, B, or C in accordance with ASTM E108 or UL 790, by an approved listing agency or shall be considered as non-classified roofing.

**2.8.3 Air and Water Infiltration:** Air infiltration resistance is outside the scope of this report. Water infiltration without underlayment is outside the scope of this report. Weather protection using underlayment for roofs, or water-resistive barrier for walls, shall comply with Section 3.0 of this report.



**2.8.4 Hail Resistance:** Hail resistance is outside the scope of this report.

**2.8.5 Wind-blown Debris Resistance:** Wind-blown debris resistance is outside the scope of this report.

**2.8.6 Fire-Resistance Rating:** Fire-resistance of walls is outside the scope of this report. Fire-resistance ratings, when required, shall be determined in accordance with IBC Section 703 or IRC Section R302.

**2.9** Mirage Panels, PBU-Panels, U-Panels, Mega-Rib Panels, Mirage II Panels, Multi-Rib Panels, and Multi-Rib PB Panels are manufactured in Adelanto, California.

### 3.0 PRODUCT USE

**3.1 Mirage Panels:** Mirage Panels comply with IBC Section 1507.4 and IRC Section R905.10 as metal roof panels and are used as new roof coverings installed over solid or closely fitted sheathing complying with the applicable code listed in Section 1.0 of this report.

**3.1.1 Design:** The allowable uniform uplift wind loads for the Mirage Panel installed in accordance with the manufacturer's installation instructions are shown in Table 1 of this report.

**3.1.2 Installation:** Roof slope shall comply with IBC Section 1507.4.2.3 or IRC Section R905.10.2.3. The underlayment shall be installed in accordance with Section 1507.1.1 and Tables 1507.1.1(1), 1507.1.1(2), and 1507.1.1(3) of the IBC or Section R905.1.1 and Tables R905.1.1(1), R905.1.1(2), and R905.1.1(3) of the IRC for metal roof panels. Mirage Panels shall be installed in a continuous run without end-laps using the Mirage clips described in Section 4.1 of this report. The fasteners used to attach the Mirage clips to the sheathing or supports shall be two No.10-16 by 1-inch-long (25.4 mm) self-drilling, corrosion-resistant galvanized steel pancake head screws per clip, spaced as described in Table 1 of this report and through the roof sheathing to steel supports of minimum No.16 gage [0.0568 inch (1.44 mm)] framing having a minimum yield strength of 55 ksi (379 MPa) and complying with the applicable code in Section 1.0 of this report. As an option, the sealant may be to the lap joints as shown in Figure 4 of this report. The sealant installation to the panel assembly shall be in accordance with the sealant manufacturer's installation instructions. Additional panel trim and accessories are provided to fit the specific needs of the job site.

**3.2 PBU-Panels and U-Panels:** PBU- and U-Panels comply with IBC Sections 1403.5, 1404.2, and 1507.4, and IRC Sections R703.3 and R905.10 as steel wall coverings and metal roof panels and are used as a new roof and wall coverings installed directly to steel framing.

**3.2.1 Design:** The section properties, determined using AISI S100 design specifications, are shown in Table 5 (U-Panel) and Table 8 (PBU-Panel), of this report.

The allowable uniform positive and negative loads for the PBU- and U-Panels, installed in accordance with this report and the manufacturer's installation instructions directly to steel framing, are shown in Tables 2 to 4; and 6 and 7, respectively, of this report.

Panel attachments shall be designed to equal or exceed the design negative loads. The design shall comply with Section J of AISI S100-16 or -16w/S1(2018) or Section E of AISI S100-12 or -07, and is subject to the approval of the building official.

The allowable horizontal and vertical diaphragm shear strengths and stiffnesses for the PBU- and U-Panels, installed in accordance with the manufacturer's installation instructions directly to steel framing, are shown in Tables 23 to 27 of this report. The tabulated values are for full-width panels. For cut panels, diaphragm shear strength and stiffness shall be evaluated in accordance with AISI S310. A load path to the foundation shall be provided for the uplift, shear, and compression forces as determined by the design professional and approved by the building official. Elements resisting shear wall forces contributed by multiple stories shall be designed for the sum of forces contributed by each story.

**3.2.2 Installation:** Roof slope shall comply with IBC Section 1507.4.2.1 or 1507.4.2.2, or IRC Section R905.10.2.1 or R905.10.2.2.

For walls, a water-resistive barrier shall be provided in accordance with IBC Sections 1403.2 and 1404.2 or IRC Section R703.2; flashing shall be provided in accordance with IBC Section 1405.4 or IRC Section R703.4.

PBU- and U-Panels shall be secured using No.12-14 by 1¼-inch long (32 mm) corrosion-resistant galvanized steel HWH self-drilling tapping screws spaced as shown in Figure 5 of this report and direct to the No.16 gage [0.0568 inch (1.44 mm)] steel supports having a minimum yield strength of 55 ksi (379 MPa) and complying with the applicable code in Section 1.0 of this report.

For installations directly over steel framing, PBU- and U-Panels shall be located in accordance with Tables 2 to 4; and 6 and 7, respectively, of this report.

The PBU-Panels and U-Panels shall be attached using galvanized steel or stainless-steel fasteners that are painted with corrosion-resistant coatings for sealant and a sealing cap for the stainless steel.

Sealant shall be applied for roof slopes of ½ units vertical in 12 units horizontal to 3 units vertical in 12 units horizontal (4 to 25 percent) to the side-seam joints, and at panel terminations. PBU- and U-Panels are installed in a continuous run with no end laps. The sealant installation to



the panel assembly shall be in accordance with the sealant manufacturer's installation instructions. Both panel ends shall be secured to the steel supports with one screw placed midway at the panel's corrugation. The fasteners being installed at the interior, have a screw installed in between alternate corrugations. In the two fastening patterns, the stitch fasteners are installed 1 foot, 8 inches (508 mm) on center at the corrugation's top at the side laps as illustrated in Figure 5 of this report. Additional panel trim and accessories are provided to fit the specific needs of the job site.

**3.3 Mega-Rib Panels:** Mega-Rib Panels comply with IBC Sections 1403.5, 1404.2, and 1507.4, and IRC Sections R703.3 and R905.10 as steel wall coverings and metal roof panels and are used as a new roof and wall coverings installed directly to steel framing.

**3.3.1 Design:** Section properties determined in accordance with AISI S100 are shown in Table 11 of this report.

The allowable uniform positive and negative loads for the Mega-Rib Panels, installed in accordance with this report and the manufacturer's installation instructions directly to steel framing, are shown in Tables 9 and 10 of this report. Panel attachments shall be designed to equal or exceed the design negative loads. The design shall comply with Section J of AISI S100-16 or -16w/S1(2018) or Section E of AISI S100-12 or -07, and is subject to the approval of the building official.

The allowable horizontal and vertical diaphragm shear strengths and stiffnesses for the Mega-Rib Panels, installed in accordance with the manufacturer's installation instructions directly to steel framing, are shown in Tables 21 and 22 of this report. The tabulated values are for full-width panels. For cut panels, diaphragm shear strength and stiffness shall be evaluated in accordance with AISI S310. A load path to the foundation shall be provided for the uplift, shear, and compression forces as determined by the design professional and approved by the building official. Elements resisting shear wall forces contributed by multiple stories shall be designed for the sum of forces contributed by each story.

**3.3.2 Installation:** Roof slope shall comply with IBC Section 1507.4.2.1 or 1507.4.2.2, or IRC Section R905.10.2.1 or R905.10.2.2. For installations directly over steel framing, Mega-Rib Panels shall be located in accordance with Tables 11 and 12 of this report.

For walls, a water-resistive barrier shall be provided in accordance with IBC 1403.2 or IRC Section R703.2; flashing shall be provided in accordance with IBC Section 1405.4 or IRC Section R703.4.

The Mega-Rib Panels shall be attached using galvanized steel or stainless-steel fasteners that are painted with corrosion-

resistant coatings for sealant and a sealing cap for the stainless steel.

Sealant shall be applied for roof slopes of  $\frac{1}{2}$  units vertical in 12 units horizontal to 3 units vertical in 12 units horizontal (4 to 25 percent) to the side-seam lap joints, and at panel terminations. The sealant installation to the panel assembly shall be in accordance with the sealant manufacturer's installation instructions. Mega-Rib Panels are installed in a continuous run with no end laps. Both panel ends shall be secured to the steel supports with one screw placed midway at the panel's corrugation. The fasteners being installed at the interior, have a screw installed in between alternate corrugations. In the two fastening patterns, the stitch fasteners are installed 1 foot, 8 inches (508 mm) on center at the corrugation's top at the side laps as illustrated in Figure 5 of this report. Additional panel trim and accessories are provided to fit the specific needs of the job site.

**3.4 Mirage II Panel:** Mirage II-Panels comply with IBC Section 1507.4, and IRC Section R905.10 as steel roof panels and are used as a new roof coverings installed directly to steel framing.

**3.4.1 Design:** The section properties, determined using AISI S100 design specifications, are shown in Table 12 of this report.

The allowable uniform positive and negative loads for the Mirage II-Panels, installed in accordance with this report and the manufacturer's installation instructions directly to steel framing, are shown in Tables 13 of this report.

Panel attachments shall be designed to equal or exceed the design negative loads. The design shall comply with Section J of AISI S100-16 and -16w/S1(2018) or Section E of AISI S100-12 or -07, and is subject to the approval of the building official.

Web crippling strengths shall equal or exceed the design positive loads. The analysis shall be in accordance with Sections G5, G6, and H3 of AISI S100-16 and -16w/S1(2018) or Sections C3.4 and C3.5 of AISI S100-12 or -07, and is subject to the approval of the building official.

**3.4.2 Installation:** Roof slope shall comply with IBC Section 1507.4.2.1 or 1507.4.2.2, or IRC Section R905.10.2.1 or R905.10.2.2.

Mirage II-Panels shall be secured to steel supports in accordance with the approved plans.

The Mirage II-Panels shall be attached using galvanized steel or stainless-steel fasteners that are painted with corrosion-resistant coatings for sealant and a sealing cap for the stainless steel.





Sealant shall be applied for roof slopes of ¼ units vertical in 12 units horizontal to 3 units vertical in 12 units horizontal (2 to 25 percent) to the side-seam joints, and at panel terminations for all roof slopes. Mirage II-Panels are installed in a continuous run with no end laps. The sealant installation to the panel assembly shall be in accordance with the sealant manufacturer's installation instructions. Additional panel trim and accessories are provided to fit the specific needs of the job site.

**3.5 Multi-Rib and Multi-Rib PB Panels:** Multi-Rib and Multi-Rib PB Panels comply with IBC Sections 1403.5, 1404.2, and 1507.4, and IRC Sections R703.3 and R905.10 as steel wall coverings and metal roof panels and are used as a new roof and wall coverings installed directly to steel framing.

**3.5.1 Design:** Section properties determined in accordance with AISI S100 are shown in Tables 14 and 18 of this report.

The allowable uniform positive and negative loads for the Multi-Rib and Multi-Rib PB Panels, installed in accordance with this report and the manufacturer's installation instructions directly to steel framing, are shown in Tables 15, 16, 17, 19 and 20 of this report. Panel attachments shall be designed to equal or exceed the design negative loads. The design shall comply with Section J of AISI S100-16 or -16w/S1(2018) or Section E of AISI S100-12 or -07, and is subject to the approval of the building official.

The allowable horizontal and vertical diaphragm shear strengths and stiffnesses for the Multi-Rib and Multi-Rib PB Panels, installed in accordance with the manufacturer's installation instructions directly to steel framing, are shown in Tables 28 to 32 of this report. The tabulated values are for full-width panels. For cut panels, diaphragm shear strength and stiffness shall be evaluated in accordance with AISI S310. A load path to the foundation shall be provided for the uplift, shear, and compression forces as determined by the design professional and approved by the building official. Elements resisting shear wall forces contributed by multiple stories shall be designed for the sum of forces contributed by each story.

**3.5.2 Installation:** Roof slope shall comply with IBC Section 1507.4.2.1 or 1507.4.2.2, or IRC Section R905.10.2.1 or R905.10.2.2. For installations directly over steel framing, Multi-Rib and Multi-Rib PB Panels shall be located in accordance with Tables 11 and 12 of this report.

For walls, a water-resistive barrier shall be provided in accordance with IBC 1403.2 or IRC Section R703.2; flashing shall be provided in accordance with IBC Section 1405.4 or IRC Section R703.4.

The Multi-Rib and Multi-Rib PB Panels shall be attached using galvanized steel or stainless-steel fasteners that are

painted with corrosion-resistant coatings for sealant and a sealing cap for the stainless steel.

Sealant shall be applied the side-seam joints and at panel terminations.. The sealant installation to the panel assembly shall be in accordance with the sealant manufacturer's installation instructions. Multi-Rib and Multi-Rib PB Panels are installed in a continuous run with no end laps. Both panel ends shall be secured to the steel supports with one screw placed midway at the panel's corrugation. The fasteners being installed at the interior, have a screw installed in between alternate corrugations. In the two fastening patterns, the stitch fasteners are installed 1 foot, 8 inches (508 mm) on center at the corrugation's top at the side laps as illustrated in Figure 5 of this report. Additional panel trim and accessories are provided to fit the specific needs of the job site.

## 4.0 PRODUCT DESCRIPTION

**4.1 Mirage Panel:** The Mirage Panel is a standing seam metal roof covering that is rolled and pressure-formed from sheet steel complying with ASTM A792, Grade 50 (Class 1 or 4) with an AZ50 or AZ55 aluminum-zinc alloy coating. The No. 24 gage panel minimum base-metal design thickness is 0.0224 inch (0.57 mm). The panel width is 16 inches (406 mm) and panel height is 1<sup>5</sup>/<sub>8</sub>-inches (41.3 mm) with 8-inch-on-center (203 mm) ribs that are equally spaced between the taller profiles. The panels are available in lengths of 3 to 50 feet (0.9 m to 15.2 m) and delivered to the job site as shown in Figure 1 of this report.

The Mirage Clip is pressure-formed steel having a No. 20 gage [0.0338-inch (0.86 mm)] minimum design base-metal thickness complying with ASTM A653 SS Grade 50 or ASTM A792 SS Grade 50.

**4.2 U-Panel:** The U-Panel is a through-fastened metal roof and wall covering that is rolled and pressure-formed from sheet steel complying with ASTM A792, Grade 50 (Class 1 or 4) or Grade 80 with an AZ50 aluminum-zinc alloy coating. The No. 24 gage panel minimum base-metal design thickness is 0.0224 inch (0.57 mm), and the 26 gage is 0.0176 inch (0.45 mm). The panel width is 36 inches (914 mm) and panel height is <sup>43</sup>/<sub>64</sub> inch (17.1 mm) with 6-inch-on-center (152 mm) continuous corrugations. The panels are available in lengths of 3 to 45 feet (0.9 m to 13.7 m) long and delivered to the job site as shown in Figure 2 of this report.

**4.3 PBU-Panel:** The PBU-Panel is identical to the U-Panel, except the PBU-Panel has a purlin-bearing edge that reinforces the corrugation's side lap as illustrated in Figure 3 of this report.

**4.4 Mega-Rib Panel:** The Mega-Rib is a through-fastened metal roof and wall covering that is rolled and pressure-formed from sheet steel complying with ASTM A792, Grade 50 (Class 1 or 4) or Grade 80 with an AZ50 aluminum-zinc alloy coating. The No. 24 gage panel minimum base-metal



design thickness is 0.0224 inch (0.57 mm), and the No. 26 gage is 0.0176 inch (0.45 mm). The panel width is 36 inches (914 mm) and its height is 1½ inches (38.1 mm) with 6-inch-on-center (152 mm) continuous corrugations. The panels are available in lengths of 3 to 40 feet (0.9 m to 13.7 m) long and delivered to the job site as shown in Figure 6 of this report.

**4.5 Mirage II Panel:** The Mirage II Panel is a standing seam metal roof covering that is rolled and pressure-formed from sheet steel complying with ASTM A792, Grade 50 (Class 1 or 4) with an AZ50 or AZ55 aluminum-zinc alloy coating. The No. 24 gage panel minimum base-metal design thickness is 0.0224 inch (0.57 mm). The panel width is 16 inches (406 mm) and panel height is 1¾-inches (44.5 mm) with 8-inch-on-center (203 mm) ribs that are equally spaced between the taller profiles. The panels are available in lengths of 3 to 40 feet (0.9 m to 13.7 m) and delivered to the job site as shown in Figure 7 of this report.

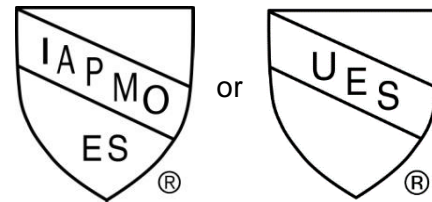
The Mirage II Clip is pressure-formed steel having a No. 18 gage [0.0478 inch (1.21 mm)] minimum design base-metal thickness complying with ASTM A653 SS Grade 50 or ASTM A792 SS Grade 50 with a G90 zinc coating.

**4.6 Multi-Rib Panel:** The Multi-Rib is a through-fastened metal roof and wall covering that is rolled and pressure-formed from sheet steel complying with ASTM A792, Grade 50 (Class 1 or 4) or Grade 80 with an AZ50 aluminum-zinc alloy coating. The No. 24 gage panel minimum base-metal design thickness is 0.0224 inch (0.57 mm), and the No. 26 gage is 0.0176 inch (0.45 mm). The panel width is 38¼ inches (972 mm) and its height is 1-3/16 inches (30.1 mm) with 12-inch-on-center (152 mm) continuous ribs, which alternate with ⅝ inch (3.2 mm) deep corrugations. The panels are available in lengths of 3 to 40 feet (0.9 m to 13.7 m) long and delivered to the job site as shown in Figure 8 of this report.

**4.7 Multi-Rib PB Panel:** The Multi is identical to the Multi-Rib, except the Multi-Rib PB width is 39⅞ inches (994 mm) and has a purlin-bearing edge that provides reinforcement in the corrugation's side lap as illustrated in Figure 8 of this report. Also, the No 26 gage panel is produced in Grade 80 only.

## 5.0 IDENTIFICATION

Mirage Panel, U-Panel, and PBU-Panel are identified with a label on the package of trim material or the pallet identifying the company name (McElroy Metal, Inc.), the product name, Adelanto, CA (Division 106), the IAPMO UES Marks of Conformity and the Evaluation Report Number (ER-270). Either Mark of Conformity may be used as shown below:



**IAPMO UES ER-270**

## 6.0 SUBSTANTIATING DATA

**6.1** Data in accordance with IAPMO Uniform ES EC 011-2019, Evaluation Criteria for Single Skin Roof and Wall Panels.

**6.2** Test reports from laboratories in compliance with ISO/IEC 17025.

**6.3** Manufacturer's descriptive literature and installation instructions.

**6.4** IAPMO Uniform ES approved Quality Control Manual, in accordance with IAPMO Uniform ES ES-10 Quality Management System Review Procedures.

## 7.0 STATEMENT OF RECOGNITION

This evaluation report describes the results of research completed by the IAPMO Uniform Evaluation Service on Mirage Panel, PBU-Panel and U-Panel manufactured in Adelanto, California to assess their conformance to the codes shown in Section 1.0 of this report and serves as documentation of the product's certification. Products are manufactured at the location noted in Section 2.9 of this report under a quality control program with periodic inspection under the surveillance program by IAPMO UES.

For additional information about this evaluation report please visit [www.uniform-es.org](http://www.uniform-es.org) or email at [info@uniform-es.org](mailto:info@uniform-es.org)



# EVALUATION REPORT

Number: 270

Originally Issued: 07/25/2016

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TABLE 1—ALLOWABLE UNIFORM UPLIFT FOR MIRAGE PANEL (psf)

MATERIAL – No. 24 gage / $F_y = 50$ ksi	
Span (ft)	Allowable Load (psf) <sup>1</sup>
1.5	92.1
2.0	86.4
2.5	80.8
3.0	75.1
3.5	69.5
4.0	63.8
4.5	58.2
5.0	52.5

For SI: 1 inch = 2.54 mm; 1 ksi = 6.89 MPa; 1 psf = 48 Pa.

<sup>1</sup>. Allowable uniform uplift loads were calculated based upon equal span lengths between clips.

TABLE 2 —ALLOWABLE UNIFORM LOADS (PSF) FOR NO. 24 GAGE U-PANEL

Span Type	Load Type	Span in Feet															
		1.50	2.00	2.50	3.00	3.50	4.00	4.50	5.00	5.50	6.00	6.50	7.00	7.50	8.00	8.50	9.00
Single	Positive Wind	455	256	164	113	83	64	50	41	33	28	24	20	18	16	14	12
	Negative Wind	407	229	146	101	74	57	45	36	30	25	21	18	16	14	12	11
	Live	455	256	164	113	83	64	50	41	33	28	24	20	18	16	14	12
	Deflection (L/180)	500	287	147	85	53	35	25	18	13	10	8	6	5	4	3	3
	Deflection (L/240)	500	215	110	63	40	26	18	13	10	7	6	5	4	3	2	2
2 Span	Positive Wind	384	221	143	100	73	56	44	36	30	25	21	18	16	14	12	11
	Negative Wind	424	246	159	111	82	63	50	40	33	28	24	20	18	15	14	12
	Live	384	221	143	100	73	56	44	36	30	25	21	18	16	14	12	11
	Deflection (L/180)	500	500	298	172	108	72	51	37	28	21	16	13	11	9	7	6
	Deflection (L/240)	500	437	223	129	81	54	38	27	21	16	12	10	8	6	5	4
3 Span	Positive Wind	469	273	177	124	92	70	56	45	37	31	26	23	20	17	15	14
	Negative Wind	500	302	197	138	102	78	62	50	42	35	30	26	22	19	17	15
	Live	469	273	177	124	92	70	56	45	37	31	26	23	20	17	15	14
	Deflection (L/180)	500	456	233	135	85	57	40	29	21	16	13	10	8	7	5	5
	Deflection (L/240)	500	342	175	101	63	42	30	21	16	12	9	7	6	5	4	3
4 Span	Positive Wind	442	256	166	116	86	66	52	42	35	29	25	21	18	16	14	13
	Negative Wind	486	284	185	129	95	73	58	47	39	33	28	24	21	18	16	14
	Live	442	256	166	116	86	66	52	42	35	29	25	21	18	16	14	13
	Deflection (L/180)	500	484	248	143	90	60	42	31	23	17	14	11	9	7	6	5
	Deflection (L/240)	500	363	186	107	67	45	31	23	17	13	10	8	6	5	4	3

For SI: 1 inch = 2.54 mm; 1 foot = 305 mm; 1 ksi = 6.89 MPa; 1 psf = 48 Pa.

## Notes:

1. Allowable uniform loads are based upon equal span lengths.
2. Live is the allowable live or snow load.
3. Deflection (L/180) is the allowable load that limits the panel's deflection to L/180 while under positive or live load.
4. Deflection (L/240) is the allowable load that limits the panel's deflection to L/240 while under positive or live load.
5. The weight of the panel has **NOT** been deducted from the allowable loads.
6. Positive Wind, Negative Wind, and Live Load values are limited to combined shear & bending using Eq. H2-1 of AISI S100-16.
7. Positive Wind and Live Load values are limited by web crippling using a minimum bearing length of 2 inches.
8. Web crippling values are determined using a ratio of the uniform load supported by the top flanges of the section.
9. Load Tables are limited to a maximum allowable load of 500 psf.
10. The deflection limits do not ensure against ponding. Roofs that deflect from snow loads or do not have sufficient slope or camber to ensure adequate drainage shall be investigated for ponding instability in accordance with Chapters 7 and 8 of ASCE 7.



# EVALUATION REPORT

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TABLE 3 —ALLOWABLE UNIFORM LOADS (PSF) FOR NO. 26 GAGE U-PANEL,  $F_y = 50$  ksi

Span Type	Load Type	Span in Feet															
		1.50	2.00	2.50	3.00	3.50	4.00	4.50	5.00	5.50	6.00	6.50	7.00	7.50	8.00	8.50	9.00
Single	Positive Wind	329	185	118	82	60	46	36	29	24	20	17	15	13	11	10	9
	Negative Wind	284	159	102	71	52	39	31	25	21	7	15	13	11	9	8	7
	Live	329	185	118	82	60	46	36	29	24	20	17	15	13	11	10	9
	Deflection (L/180)	499	210	107	62	39	26	18	13	10	7	6	4	3	3	2	2
	Deflection (L/240)	374	158	80	46	29	19	13	10	7	5	4	3	2	2	2	1
2 Span	Positive Wind	284	159	102	71	52	39	31	25	21	17	15	13	11	9	8	7
	Negative Wind	329	185	118	82	60	46	36	29	24	20	17	15	13	11	10	9
	Live	284	159	102	71	52	39	31	25	21	17	15	13	11	9	8	7
	Deflection (L/180)	500	425	217	125	79	53	37	27	20	15	12	9	8	6	5	4
	Deflection (L/240)	500	318	163	94	59	39	27	20	15	11	9	7	6	4	4	3
3 Span	Positive Wind	355	199	127	88	65	49	39	31	26	22	18	16	14	12	11	9
	Negative Wind	411	231	148	102	75	57	45	37	30	25	21	18	16	14	12	11
	Live	355	199	127	88	65	49	39	31	26	22	18	16	14	12	11	9
	Deflection (L/180)	500	332	170	98	62	41	29	21	16	12	9	7	6	5	4	3
	Deflection (L/240)	500	249	127	73	46	31	21	15	12	9	7	5	4	3	3	2
4 Span	Positive Wind	331	186	119	82	60	46	36	29	24	20	17	15	13	11	10	9
	Negative Wind	384	216	138	96	70	54	42	34	28	24	20	17	15	13	11	10
	Live	331	186	119	82	60	46	36	29	24	20	17	15	13	11	10	9
	Deflection (L/180)	500	353	180	104	65	44	31	22	16	13	10	8	6	5	4	3
	Deflection (L/240)	500	265	135	78	49	33	23	16	12	9	7	6	5	4	3	2

For SI: 1 inch = 25.4 mm; 1 foot = 305 mm; 1 ksi = 6.89 MPa; 1 psf = 48 Pa.

#### Notes:

1. Allowable uniform loads are based upon equal span lengths.
2. Live is the allowable live or snow load.
3. Deflection (L/180) is the allowable load that limits the panel's deflection to L/180 while under positive or live load.
4. Deflection (L/240) is the allowable load that limits the panel's deflection to L/240 while under positive or live load.
5. The weight of the panel has **NOT** been deducted from the allowable loads.
6. Positive Wind, Negative Wind, and Live Load values are limited to combined shear & bending using Eq. H2-1 of AISI S100-16.
7. Positive Wind and Live Load values are limited by web crippling using a minimum bearing length of 2 inches.
8. Web crippling values are determined using a ratio of the uniform load supported by the top flanges of the section.
9. Load Tables are limited to a maximum allowable load of 500 psf.
10. The deflection limits do not ensure against ponding. Roofs that deflect from snow loads or do not have sufficient slope or camber to ensure adequate drainage shall be investigated for ponding instability in accordance with Chapters 7 and 8 of ASCE 7.



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TABLE 4—ALLOWABLE UNIFORM LOADS (PSF) FOR NO. 26 GAGE U-PANEL,  $F_y = 80$  ksi

Span Type	Load Type	Span in Feet															
		1.50	2.00	2.50	3.00	3.50	4.00	4.50	5.00	5.50	6.00	6.50	7.00	7.50	8.00	8.50	9.00
Single	Positive Wind	367	206	132	91	67	51	40	33	27	22	19	16	14	12	11	10
	Negative Wind	313	176	112	78	57	44	34	28	23	19	16	14	12	11	9	8
	Live	367	206	132	91	67	51	40	33	27	22	19	16	14	12	11	10
	Deflection (L/180)	466	196	100	58	36	24	17	12	9	7	5	4	3	3	2	2
	Deflection (L/240)	349	147	75	43	27	18	12	9	7	5	4	3	2	2	1	1
2 Span	Positive Wind	299	171	110	77	57	43	34	28	23	19	16	14	12	10	9	8
	Negative Wind	345	199	129	90	66	51	40	32	27	22	19	16	14	12	11	10
	Live	299	171	110	77	57	43	34	28	23	19	16	14	12	10	9	8
	Deflection (L/180)	500	399	204	118	74	49	35	25	19	14	11	9	7	6	5	4
	Deflection (L/240)	500	299	153	88	55	37	26	19	14	11	8	6	5	4	3	3
3 Span	Positive Wind	367	212	137	96	71	54	43	35	28	24	20	17	15	13	12	10
	Negative Wind	421	245	160	112	82	63	50	41	33	28	24	21	18	16	14	12
	Live	367	212	137	96	71	54	43	35	28	24	20	17	15	13	12	10
	Deflection (L/180)	500	312	160	92	58	39	27	20	15	11	9	7	5	4	4	3
	Deflection (L/240)	500	234	120	69	43	29	20	15	11	8	6	5	4	3	3	2
4 Span	Positive Wind	345	198	128	90	66	50	40	32	27	22	19	16	14	12	11	10
	Negative Wind	396	230	149	105	77	59	47	38	31	26	22	19	17	15	13	11
	Live	345	198	128	90	66	50	40	32	27	22	19	16	14	12	11	10
	Deflection (L/180)	500	331	169	98	61	41	29	21	15	12	9	7	6	5	4	3
	Deflection (L/240)	500	248	127	73	46	31	21	15	11	9	7	5	4	3	3	2

For SI: 1 inch = 2.54 mm; 1 foot = 305 mm; 1 ksi = 6.89 MPa; 1 psf = 48 Pa.

#### Notes:

1. Allowable uniform loads are based upon equal span lengths.
2. Live is the allowable live or snow load.
3. Deflection (L/180) is the allowable load that limits the panel's deflection to L/180 while under positive or live load.
4. Deflection (L/240) is the allowable load that limits the panel's deflection to L/240 while under positive or live load.
5. The weight of the panel has **NOT** been deducted from the allowable loads.
6. Positive Wind, Negative Wind, and Live Load values are limited to combined shear & bending using Eq. H2-1 of AISI S100-16.
7. Positive Wind and Live Load values are limited by web crippling using a minimum bearing length of 2 inches.
8. Web crippling values are determined using a ratio of the uniform load supported by the top flanges of the section.
9. Load Tables are limited to a maximum allowable load of 500 psf.
10. The deflection limits do not ensure against ponding. Roofs that deflect from snow loads or do not have sufficient slope or camber to ensure adequate drainage shall be investigated for ponding instability in accordance with Chapters 7 and 8 of ASCE 7.

TABLE 5—EFFECTIVE SECTION PROPERTIES FOR U-PANEL

SECTION PROPERTIES						TOP IN COMPRESSION			BOTTOM IN COMPRESSION		
GAGE	$F_y$ (ksi)	WEIGHT (psf)	$V_a$ (kip/ft)	$P_{a\_end}$ (lbf/ft)	$P_{a\_int}$ (lbf/ft)	$I_x$ (in <sup>4</sup> /ft)	$S_e$ (in <sup>3</sup> /ft)	$M_a$ (kip-in/ft)	$I_x$ (in <sup>4</sup> /ft)	$S_e$ (in <sup>3</sup> /ft)	$M_a$ (kip-in/ft)
24	50	1.10	1.0990	426.67	629.13	0.0263	0.0514	1.5387	0.018	0.0459	1.3747
26	50	0.87	0.8653	276.27	401.1	0.0193	0.0371	1.112	0.013	0.032	0.9593
26	80	0.85	0.9537	314.67	455.97	0.0180	0.0345	1.241	0.0123	0.0294	1.0573
29	80	0.67	0.5857	202.03	287.8	0.0127	0.0235	0.846	0.009	0.0208	0.747

For SI: 1 inch = 2.54 mm; 1 foot = 305 mm; 1 ksi = 6.89 MPa; 1 psf = 48 Pa; 1 lbf = 4.45 N; 1 kip = 4.45 kN; 1 in<sup>3</sup> = 16.39 cm<sup>3</sup>; 1 in<sup>4</sup> = 41. 62 cm<sup>4</sup>; 1 kip-in = 11.30 kN-cm.

1. Section properties are calculated in accordance with the AISI S100-16.
2.  $V_a$  is the allowable shear.
3.  $P_a$  is the allowable load for web crippling on end and interior supports using a bearing length of 2 inches.
4.  $I_x$  is for deflection determination.
5.  $S_e$  is for bending.
6.  $M_a$  is the allowable bending moment.
7. All values are for one foot of panel width.





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TABLE 6 — ALLOWABLE UNIFORM LOADS (PSF) FOR NO. 24 GAGE U-PANEL PB

Span Type	Load Type	Span in Feet															
		1.50	2.00	2.50	3.00	3.50	4.00	4.50	5.00	5.50	6.00	6.50	7.00	7.50	8.00	8.50	9.00
Single	Positive Wind	462	260	166	115	84	65	51	41	34	28	24	21	18	16	14	12
	Negative Wind	406	228	146	101	74	57	45	36	30	25	21	18	16	14	12	11
	Live	462	260	166	115	84	65	51	41	34	28	24	21	18	16	14	12
	Deflection (L/180)	500	291	149	86	54	36	25	18	14	10	8	6	5	4	3	3
	Deflection (L/240)	500	218	111	64	40	27	19	13	10	8	6	5	4	3	2	2
2 Span	Positive Wind	387	222	143	100	74	56	44	36	30	25	21	18	16	14	12	11
	Negative Wind	434	250	162	113	83	64	50	41	34	28	24	21	18	16	14	12
	Live	387	222	143	100	74	56	44	36	30	25	21	18	16	14	12	11
	Deflection (L/180)	500	500	305	176	111	74	52	38	28	22	17	13	11	9	7	6
	Deflection (L/240)	500	447	229	132	83	55	39	28	21	16	13	10	8	6	5	4
3 Span	Positive Wind	474	274	178	124	92	70	56	45	37	31	26	23	20	17	15	14
	Negative Wind	500	308	201	141	104	80	63	51	42	35	30	26	23	20	17	16
	Live	474	274	178	124	92	70	56	45	37	31	26	23	20	17	15	14
	Deflection (L/180)	500	467	239	138	87	58	41	29	22	17	13	10	8	7	6	5
	Deflection (L/240)	500	350	179	103	65	43	30	22	16	12	10	8	6	5	4	3
4 Span	Positive Wind	446	257	167	116	86	66	52	42	35	29	25	21	18	16	14	13
	Negative Wind	498	289	188	132	97	74	59	48	39	33	28	24	21	18	16	14
	Live	446	257	167	116	86	66	52	42	35	29	25	21	18	16	14	13
	Deflection (L/180)	500	496	253	146	92	62	43	31	23	18	14	11	9	7	6	5
	Deflection (L/240)	500	372	190	110	69	46	32	23	17	13	10	8	7	5	4	4

For SI: 1 inch = 2.54 mm; 1 foot = 305 mm; 1 ksi = 6.89 MPa; 1 psf = 48 Pa.

1. Allowable uniform loads are based upon equal span lengths.
2. Live is the allowable live or snow load.
3. Deflection (L/180) is the allowable load that limits the panel's deflection to L/180 while under positive or live load.
4. Deflection (L/240) is the allowable load that limits the panel's deflection to L/240 while under positive or live load.
5. The weight of the panel has **NOT** been deducted from the allowable loads.
6. Positive Wind, Negative Wind, and Live Load values are limited to combined shear & bending using Eq. H2-1 of AISI S100-16.
7. Positive Wind and Live Load values are limited by web crippling using a minimum bearing length of 2 inches.
8. Web crippling values are determined using a ratio of the uniform load supported by the top flanges of the section.
9. Load Tables are limited to a maximum allowable load of 500 psf.
10. The deflection limits do not ensure against ponding. Roofs that deflect from snow loads or do not have sufficient slope or camber to ensure adequate drainage shall be investigated for ponding instability in accordance with Chapters 7 and 8 of ASCE 7.



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TABLE 7 — ALLOWABLE UNIFORM LOADS (PSF) FOR NO. 26 GAGE U-PANEL PB,  $F_y = 80$  ksi

Span Type	Load Type	Span in Feet															
		1.50	2.00	2.50	3.00	3.50	4.00	4.50	5.00	5.50	6.00	6.50	7.00	7.50	8.00	8.50	9.00
Single	Positive Wind	389	218	140	97	71	54	43	35	28	24	20	17	15	13	12	10
	Negative Wind	329	185	118	82	60	46	36	29	24	20	17	15	13	11	10	9
	Live	389	218	140	97	71	54	43	35	28	24	20	17	15	13	12	10
	Deflection (L/180)	500	211	108	62	39	26	18	13	10	7	6	4	4	3	2	2
	Deflection (L/240)	375	158	81	46	29	19	13	10	7	5	4	3	3	2	2	1
2 Span	Positive Wind	317	181	116	81	60	46	36	29	24	20	17	15	13	11	10	9
	Negative Wind	369	212	137	95	70	54	42	34	28	24	20	17	15	13	12	10
	Live	317	181	116	81	60	46	36	29	24	20	17	15	13	11	10	9
	Deflection (L/180)	500	429	220	127	80	53	37	27	20	15	12	10	8	6	5	4
	Deflection (L/240)	500	322	165	95	60	40	28	20	15	11	9	7	6	5	4	3
3 Span	Positive Wind	390	224	145	101	74	57	45	36	30	25	21	18	16	14	12	11
	Negative Wind	451	262	170	119	88	67	53	43	35	30	25	22	19	17	15	13
	Live	390	224	145	101	74	57	45	36	30	25	21	18	16	14	12	11
	Deflection (L/180)	500	336	172	99	62	42	29	21	16	12	9	7	6	5	4	3
	Deflection (L/240)	500	252	129	74	47	31	22	16	12	9	7	5	4	3	3	2
4 Span	Positive Wind	366	210	135	94	69	53	42	34	28	23	20	17	15	13	11	10
	Negative Wind	424	245	159	111	82	63	50	40	33	28	24	20	18	15	14	12
	Live	366	210	135	94	69	53	42	34	28	23	20	17	15	13	11	10
	Deflection (L/180)	500	357	182	105	66	44	31	22	17	13	10	8	6	5	4	3
	Deflection (L/240)	500	268	137	79	50	33	23	17	12	9	7	6	5	4	3	2

For SI: 1 inch = 25.4 mm; 1 foot = 305 mm; 1 ksi = 6.89 MPa; 1 psf = 48 Pa.

#### Notes:

1. Allowable uniform loads are based upon equal span lengths.
2. Live is the allowable live or snow load.
3. Deflection (L/180) is the allowable load that limits the panel's deflection to L/180 while under positive or live load.
4. Deflection (L/240) is the allowable load that limits the panel's deflection to L/240 while under positive or live load.
5. The weight of the panel has **NOT** been deducted from the allowable loads.
6. Positive Wind, Negative Wind, and Live Load values are limited to combined shear & bending using Eq. H2-1 of AISI S100-16.
7. Positive Wind and Live Load values are limited by web crippling using a minimum bearing length of 2 inches.
8. Web crippling values are determined using a ratio of the uniform load supported by the top flanges of the section.
9. Load Tables are limited to a maximum allowable load of 500 psf.
10. The deflection limits do not ensure against ponding. Roofs that deflect from snow loads or do not have sufficient slope or camber to ensure adequate drainage shall be investigated for ponding instability in accordance with Chapters 7 and 8 of ASCE 7.



TABLE 8—EFFECTIVE SECTION PROPERTIES FOR PBU-PANEL

SECTION PROPERTIES						TOP IN COMPRESSION			BOTTOM IN COMPRESSION		
GAGE	$F_y$ (ksi)	WEIGHT (psf)	$V_a$ (lb/ft)	$P_{a\_end}$ (lb/ft)	$P_{a\_int}$ (lb/ft)	$I_x$ (in <sup>4</sup> /ft)	$S_e$ (in <sup>3</sup> /ft)	$M_a$ (kip-in/ft)	$I_x$ (in <sup>4</sup> /ft)	$S_e$ (in <sup>3</sup> /ft)	$M_a$ (kip-in/ft)
24	50	1.13	1.1933	463.47	683.4	0.0267	0.0522	1.56	0.0187	0.0459	1.3733
26	50	0.90	0.9087	300.17	435.67	0.0197	0.0377	1.13	0.0133	0.0336	1.0067
26	80	0.90	1.0973	360.1	522.8	0.0193	0.0365	1.313	0.0133	0.0309	1.117
29	80	0.71	0.6353	219.6	312.6	0.013	0.024	0.8613	0.0093	0.0219	0.787

For SI: 1 inch = 2.54 mm; 1 foot = 305 mm; 1 ksi = 6.89 MPa; 1 psf = 48 Pa; 1 lbf = 4.45 N; 1 kip = 4.45 kN; 1 in<sup>3</sup> = 16.39 cm<sup>3</sup>; 1 in<sup>4</sup> = 41. 62 cm<sup>4</sup>; 1 kip-in = 11.30 kN-cm.

Notes:

1. Section properties are calculated in accordance with AISI S100-16.
2.  $V_a$  is the allowable shear.
3.  $P_a$  is the allowable load for web crippling on end and interior supports using a bearing length of 2 inches (5.08 mm).
4.  $I_x$  is for deflection determination.
5.  $S_e$  is for bending.
6.  $M_a$  is the allowable bending moment.
7. All values are for one foot of panel width.

TABLE 9 — ALLOWABLE UNIFORM LOADS (PSF) FOR NO. 24 GAGE MEGA-RIB PANEL

		Span in Feet															
Span	Load Type	3.50	4.00	4.50	5.00	5.50	6.00	6.50	7.00	7.50	8.00	8.50	9.00	9.50	10.00	10.50	11.00
Single	Positive Wind	149	114	90	73	60	50	43	37	32	28	25	22	20	18	16	15
	Negative Wind	132	101	80	64	53	45	38	33	28	25	22	20	17	16	14	13
	Live	149	114	90	73	60	50	43	37	32	28	25	22	20	18	16	15
	Deflection (L/180)	185	124	87	63	47	36	28	23	18	15	12	10	9	7	6	5
	Deflection (L/240)	139	93	65	47	35	27	21	17	14	11	9	8	6	5	5	4
2 Span	Positive Wind	128	99	78	63	52	44	38	32	28	25	22	19	17	16	14	13
	Negative Wind	144	111	88	71	59	50	42	37	32	28	25	22	20	18	16	15
	Live	128	99	78	63	52	44	38	32	28	25	22	19	17	16	14	13
	Deflection (L/180)	441	296	207	151	113	87	69	55	44	37	30	25	22	18	16	14
	Deflection (L/240)	331	222	155	113	85	65	51	41	33	27	23	19	16	14	12	10
3 Span	Positive Wind	159	123	97	79	65	55	47	40	35	31	27	24	22	20	18	16
	Negative Wind	178	137	109	89	74	62	53	46	40	35	31	28	25	22	20	18
	Live	159	123	97	79	65	55	47	40	35	31	27	24	22	20	18	16
	Deflection (L/180)	346	231	162	118	89	68	54	43	35	28	24	20	17	14	12	11
	Deflection (L/240)	259	173	122	89	66	51	40	32	26	21	18	15	12	11	9	8
4 Span	Positive Wind	149	115	91	74	61	51	44	38	33	29	26	23	20	18	17	15
	Negative Wind	167	129	102	83	69	58	49	43	37	33	29	26	23	21	19	17
	Live	149	115	91	74	61	51	44	38	33	29	26	23	20	18	17	15
	Deflection (L/180)	367	246	172	126	94	72	57	45	37	30	25	21	18	15	13	11
	Deflection (L/240)	275	184	129	94	71	54	43	34	28	23	19	16	13	11	10	8

For SI: 1 inch = 2.54 mm; 1 ksi = 6.89 MPa; 1 psf = 48 Pa.

Notes:

1. Allowable uniform loads are based upon equal span lengths.
2. Live is the allowable live or snow load.
3. Deflection (L/180) is the allowable load that limits the panel's deflection to L/180 while under positive or live load.
4. Deflection (L/240) is the allowable load that limits the panel's deflection to L/240 while under positive or live load.
5. The weight of the panel has NOT been deducted from the allowable loads.
6. Positive Wind, Negative Wind, and Live Load values are limited to combined shear and bending using Eq. H2-1 of AISI S100-16.
7. Positive Wind and Live Load values are limited by web crippling using a minimum bearing length of 3/4 inches.
8. Web crippling values are determined using a ratio of the uniform load supported by the top flanges of the section.
9. Web crippling values are determined from ASTM E72-98 testing.
10. Web crippling values are the more conservative value whether from analytical calculation or testing.
11. Load Tables are limited to a maximum allowable load of 500 psf.
12. The deflection limits do not ensure against ponding. Roofs that deflect from snow loads or do not have sufficient slope or camber to ensure adequate drainage shall be investigated for ponding instability in accordance with Chapters 7 and 8 of ASCE 7.



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TABLE 10 —ALLOWABLE UNIFORM LOADS (PSF) FOR NO. 26 GAGE MEGA-RIB PANEL

Span	Load Type	Span in Feet															
		3.50	4.00	4.50	5.00	5.50	6.00	6.50	7.00	7.50	8.00	8.50	9.00	9.50	10.00	10.50	11.00
Single	Positive Wind	110	84	66	54	44	37	32	27	24	21	18	16	14	13	12	11
	Negative Wind	93	71	56	46	38	31	27	23	20	17	15	14	12	11	10	9
	Live	110	84	66	54	44	37	32	27	24	21	18	16	14	13	12	11
	Deflection (L/180)	124	83	58	42	32	24	19	15	12	10	8	7	6	5	4	4
	Deflection (L/240)	93	62	44	32	24	18	14	11	9	7	6	5	4	4	3	3
2 Span	Positive Wind	90	69	55	45	37	31	26	23	20	17	15	14	12	11	10	9
	Negative Wind	104	80	64	52	43	36	31	27	23	20	18	16	14	13	12	11
	Live	90	69	55	45	37	31	26	23	20	17	15	14	12	11	10	9
	Deflection (L/180)	292	195	137	100	75	57	45	36	29	24	20	17	14	12	10	9
	Deflection (L/240)	219	146	103	75	56	43	34	27	22	18	15	12	10	9	8	7
3 Span	Positive Wind	110	85	68	55	46	39	33	28	25	22	19	17	15	14	12	11
	Negative Wind	127	99	79	64	54	45	39	33	29	25	23	20	18	16	15	13
	Live	110	85	68	55	46	39	33	28	25	22	19	17	15	14	12	11
	Deflection (L/180)	228	153	107	78	58	45	35	28	23	19	15	13	11	9	8	7
	Deflection (L/240)	171	114	80	58	44	34	26	21	17	14	11	10	8	7	6	5
4 Span	Positive Wind	103	80	64	52	43	36	31	27	23	20	18	16	14	13	12	11
	Negative Wind	119	93	74	60	50	42	36	31	27	24	21	19	17	15	14	12
	Live	103	80	64	52	43	36	31	27	23	20	18	16	14	13	12	11
	Deflection (L/180)	242	162	114	83	62	48	37	30	24	20	16	14	12	10	8	7
	Deflection (L/240)	182	122	85	62	46	36	28	22	18	15	12	10	9	7	6	5

For SI: 1 inch = 2.54 mm; 1 ksi = 6.89 MPa; 1 psf = 48 Pa.

Notes:

1. Allowable uniform loads are based upon equal span lengths.
2. Live is the allowable live or snow load.
3. Deflection (L/180) is the allowable load that limits the panel's deflection to L/180 while under positive or live load.
4. Deflection (L/240) is the allowable load that limits the panel's deflection to L/240 while under positive or live load.
5. The weight of the panel has **NOT** been deducted from the allowable loads.
6. Positive Wind, Negative Wind, and Live Load values are limited to combined shear & bending using Eq. H2-1 of AISI S100-16.
7. Positive Wind and Live Load values are limited by web crippling using a minimum bearing length of 2 inches.
8. Web crippling values are determined using a ratio of the uniform load supported by the top flanges of the section.
9. The deflection limits do not ensure against ponding. Roofs that deflect from snow loads or do not have sufficient slope or camber to ensure adequate drainage shall be investigated for ponding instability in accordance with Chapters 7 and 8 of ASCE 7.

TABLE 11—EFFECTIVE SECTION PROPERTIES FOR MEGA-RIB PANEL

SECTION PROPERTIES						TOP IN COMPRESSION			BOTTOM IN COMPRESSION		
GAGE	F <sub>y</sub> (ksi)	WEIGHT (psf)	V <sub>a</sub> (kip/ft.)	Pa_end (lbs/ft.)	Pa_int (lbs/ft.)	I <sub>x</sub> (in <sup>4</sup> /ft.)	S <sub>e</sub> (in <sup>3</sup> /ft.)	M <sub>a</sub> (kip-in./ft.)	I <sub>x</sub> (in <sup>4</sup> /ft.)	S <sub>e</sub> (in <sup>3</sup> /ft.)	M <sub>a</sub> (kip-in./ft.)
24	50.0	1.17	1.2580	233.13	643.31	0.0910	0.1098	2.7433	0.0890	0.0973	2.4300
26	80.0	0.93	0.6927	244.73	294.63	0.0613	0.06763	2.0287	0.0577	0.0575	1.7257

For SI: 1 inch = 2.54 mm; 1 foot = 305 mm; 1 ksi = 6.89 MPa; 1 psf = 48 Pa; 1 lbf = 4.45 N; 1 kip = 4.45 kN; 1 in<sup>3</sup> = 16.39 cm<sup>3</sup>; 1 in<sup>4</sup> = 41. 62 cm<sup>4</sup>; 1 kip-in = 11.30 kN-cm.

Notes:

1. Section properties are calculated in accordance with the AISI S100-16.
2. V<sub>a</sub> is the allowable shear.
3. P<sub>a</sub> is the allowable load for web crippling on end and interior supports using a minimum bearing length of 3½ inches for No. 24 gage and 2 inches for No. 26 gage.
4. I<sub>x</sub> is for deflection determination.
5. S<sub>e</sub> is for bending.
6. M<sub>a</sub> is the allowable bending moment.
7. All values are for one foot of panel width.





TABLE 12—EFFECTIVE SECTION PROPERTIES FOR MIRAGE II PANEL

SECTION PROPERTIES				TOP IN COMPRESSION			BOTTOM IN COMPRESSION		
GAGE	F <sub>y</sub> (KSI)	WEIGHT (PSF)	V <sub>a</sub> kip/ft.	I <sub>x</sub> (in. <sup>4</sup> /ft.)	S <sub>e</sub> (in. <sup>3</sup> /ft.)	M <sub>a</sub> kip-in./ft.	I <sub>x</sub> (in. <sup>4</sup> /ft.)	S <sub>e</sub> (in. <sup>3</sup> /ft.)	M <sub>a</sub> kip-in./ft.
24	50.0	1.42	1.5030	0.1530	0.1193	3.5690	0.1350	0.2300	3.4200

For SI: 1 inch = 25.4 mm; 1 foot = 305 mm; 1 ksi = 6.89 MPa; 1 psf = 48 Pa; 1 lbf = 4.45 N; 1 kip = 4.45 kN; 1 in<sup>3</sup> = 16.39 cm<sup>3</sup>; 1 in<sup>4</sup> = 41.62 cm<sup>4</sup>; 1 kip-in = 11.30 kN-cm.

Notes:

1. Section properties are calculated in accordance with AISI S100-16.
2. V<sub>a</sub> is the allowable shear.
3. I<sub>x</sub> is for deflection determination.
4. S<sub>e</sub> is for bending.
5. M<sub>a</sub> is the allowable bending moment.
6. All values are for one foot of panel width.

TABLE 13—ALLOWABLE UNIFORM LOADS (PSF) FOR MIRAGE II PANEL

		Span in Feet															
Span Type	Load Type	1.00	1.50	2.00	2.50	3.00	3.50	4.00	4.50	5.00	5.50	6.00	6.50	7.00	7.50	8.00	8.50
Single	Positive Wind	500	500	500	380	264	194	148	117	95	78	66	56	48	42	37	32
	Live	500	500	500	380	264	194	148	117	95	78	66	56	48	42	37	32
	Deflection (L/180)	500	500	500	500	495	311	208	146	106	80	61	48	38	31	26	21
	Deflection (L/240)	500	500	500	500	371	233	156	110	80	60	46	36	29	23	19	16
2 Span	Positive Wind	500	500	500	341	241	179	138	110	89	74	62	53	46	40	35	31
	Live	500	500	500	341	241	179	138	110	89	74	62	53	46	40	35	31
	Deflection (L/180)	500	500	500	500	500	500	473	332	242	182	140	110	88	71	59	49
	Deflection (L/240)	500	500	500	500	500	500	355	249	181	136	105	82	66	53	44	37
3 Span	Positive Wind	500	500	500	415	296	221	171	136	111	92	77	66	57	50	44	39
	Live	500	500	500	415	296	221	171	136	111	92	77	66	57	50	44	39
	Deflection (L/180)	500	500	500	500	500	500	371	260	190	142	109	86	69	56	46	38
	Deflection (L/240)	500	500	500	500	500	415	278	195	142	107	82	64	51	42	34	29
4 Span	Positive Wind	500	500	500	391	278	207	160	127	104	86	72	62	53	46	41	36
	Live	500	500	500	391	278	207	160	127	104	86	72	62	53	46	41	36
	Deflection (L/180)	500	500	500	500	500	500	393	276	201	151	116	91	73	59	49	41
	Deflection (L/240)	500	500	500	500	500	441	295	207	151	113	87	68	55	44	36	30
ASTM E1592 Wind Uplift Testing		64.1	59.3	55.6	51.8	48.1	44.4	40.6	36.9	33.2							

For SI: 1 inch = 25.4 mm; 1 ksi = 6.89 MPa; 1 psf = 48 Pa.

Notes:

1. Allowable uniform loads are based upon equal span lengths.
2. Live is the allowable live or snow load.
3. Deflection (L/180) is the allowable load that limits the panel's deflection to L/180 while under positive or live load.
4. Deflection (L/240) is the allowable load that limits the panel's deflection to L/240 while under positive or live load.
5. The weight of the panel has **NOT** been deducted from the allowable loads.
6. Positive Wind and Live Load values are limited to combined shear and bending using Eq. H2-1 of AISI S100-16.
7. Values from ASTM E1592 Wind Uplift Testing include a factor of safety of 1.67 at 1'-0" span. A factor of safety of 1.7 is used otherwise. Shaded areas are outside of test range.
8. Web crippling has NOT been checked for this panel.
9. Load Tables are limited to a maximum allowable load of 500 psf.
10. ASTM E1592 Uplift values are not limited by anchorage or supports. The anchorage shall be analyzed and calculated for each support thickness/fastener type.
11. The deflection limits do not ensure against ponding. Roofs that deflect from snow loads or do not have sufficient slope or camber to ensure adequate drainage shall be investigated for ponding instability in accordance with Chapters 7 and 8 of ASCE 7.



TABLE 14—EFFECTIVE SECTION PROPERTIES FOR MULTI-RIB PANEL

SECTION PROPERTIES						TOP IN COMPRESSION			BOTTOM IN COMPRESSION		
GAGE	F <sub>y</sub> (KSI)	WEIGHT (PSF)	V <sub>a</sub> kip/ft.	P <sub>a_end</sub> lbs/ft.	P <sub>a_int</sub> lbs/ft.	I <sub>x</sub> (in. <sup>4</sup> /ft.)	S <sub>e</sub> (in. <sup>3</sup> /ft.)	M <sub>a</sub> kip-in./ft.	I <sub>x</sub> (in. <sup>4</sup> /ft.)	S <sub>e</sub> (in. <sup>3</sup> /ft.)	M <sub>a</sub> kip-in./ft.
24	50.0	1.10	0.7727	235.0	280.7	0.050	0.055	1.375	0.029	0.046	1.148
26	50.0	0.87	0.5407	150.7	174.3	0.035	0.037	0.937	0.022	0.033	0.821
26	80.0	0.85	0.5590	171.4	197.4	0.032	0.034	1.011	0.020	0.030	0.911

For SI: 1 inch = 2.54 mm; 1 foot = 305 mm; 1 ksi = 6.89 MPa; 1 psf = 48 Pa; 1 lbf = 4.45 N; 1 kip = 4.45 kN; 1 in<sup>3</sup> = 16.39 cm<sup>3</sup>; 1 in<sup>4</sup> = 41. 62 cm<sup>4</sup>; 1 kip-in = 11.30 kN-cm.

Notes:

1. Section properties are calculated in accordance with AISI S100-16.
2. V<sub>a</sub> is the allowable shear.
3. P<sub>a</sub> is the allowable load for web crippling on end and interior supports using a bearing length of 2 inches (5.08 mm).
4. I<sub>x</sub> is for deflection determination.
5. S<sub>e</sub> is for bending.
6. M<sub>a</sub> is the allowable bending moment.
7. All values are for one foot of panel width.

TABLE 15 — ALLOWABLE UNIFORM LOADS (PSF) FOR NO. 24 GAGE MULTI-RIB PANEL

		Span in Feet															
Span Type	Load Type	1.50	2.00	2.50	3.00	3.50	4.00	4.50	5.00	5.50	6.00	6.50	7.00	7.50	8.00	8.50	9.00
Single	Positive Wind	407	229	146	101	74	57	45	36	30	25	21	18	16	14	12	11
	Negative Wind	340	191	122	85	62	47	37	30	25	21	18	15	13	11	10	9
	Live	407	229	146	101	74	57	45	36	30	25	21	18	16	14	12	11
	Deflection (L/180)	500	500	279	161	101	68	47	34	26	20	15	12	10	8	7	5
	Deflection (L/240)	500	409	209	121	76	51	35	26	19	15	11	9	7	6	5	4
2 Span	Positive Wind	314	182	118	83	61	47	37	30	25	21	18	15	13	11	10	9
	Negative Wind	365	214	140	98	73	56	44	36	30	25	21	18	16	14	12	11
	Live	314	182	118	83	61	47	37	30	25	21	18	15	13	11	10	9
	Deflection (L/180)	500	500	500	308	193	129	91	66	49	38	30	24	19	16	13	11
	Deflection (L/240)	500	500	399	231	145	97	68	49	37	28	22	18	14	12	10	8
3 Span	Positive Wind	381	224	146	103	76	58	46	37	31	26	22	19	16	14	13	11
	Negative Wind	437	261	172	122	90	69	55	45	37	31	26	23	20	17	15	14
	Live	381	224	146	103	76	58	46	37	31	26	22	19	16	14	13	11
	Deflection (L/180)	500	500	416	241	151	101	71	52	39	30	23	18	15	12	10	8
	Deflection (L/240)	500	500	312	180	113	76	53	39	29	22	17	14	11	9	7	6
4 Span	Positive Wind	359	210	137	96	71	54	43	35	29	24	21	18	15	13	12	10
	Negative Wind	414	246	162	114	84	65	51	42	34	29	25	21	18	16	14	13
	Live	359	210	137	96	71	54	43	35	29	24	21	18	15	13	12	10
	Deflection (L/180)	500	500	442	256	161	108	75	55	41	32	25	20	16	13	11	9
	Deflection (L/240)	500	500	331	192	120	81	56	41	31	24	18	15	12	10	8	7

For SI: 1 inch = 2.54 mm; 1 foot = 305 mm; 1 ksi = 6.89 MPa; 1 psf = 48 Pa.

Notes:

1. Allowable uniform loads are based upon equal span lengths.
2. Live is the allowable live or snow load.
3. Deflection (L/180) is the allowable load that limits the panel's deflection to L/180 while under positive or live load.
4. Deflection (L/240) is the allowable load that limits the panel's deflection to L/240 while under positive or live load.
5. The weight of the panel has **NOT** been deducted from the allowable loads.
6. Positive Wind, Negative Wind, and Live Load values are limited to combined shear & bending using Eq. H2-1 of the AISI S100.
7. Positive Wind and Live Load values are limited by web crippling using a bearing length of 2 inches.
8. Web crippling values are determined using a ratio of the uniform load **actually** supported by the top flanges of the section.
9. Load Tables are limited to a maximum allowable load of 500 psf.
10. The deflection limits do not ensure against ponding. Roofs that deflect from snow loads or do not have sufficient slope or camber to ensure adequate drainage shall be investigated for ponding instability in accordance with Chapters 7 and 8 of ASCE 7.



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TABLE 16 — ALLOWABLE UNIFORM LOADS (PSF) FOR NO. 26 GAGE MULTI-RIB PANEL,  $F_y = 50$  ksi

Span Type	Load Type	Span in Feet															
		1.50	2.00	2.50	3.00	3.50	4.00	4.50	5.00	5.50	6.00	6.50	7.00	7.50	8.00	8.50	9.00
Single	Positive Wind	277	156	99	69	50	39	30	24	20	17	14	12	11	9	8	7
	Negative Wind	243	136	87	60	44	34	27	21	18	15	12	11	9	8	7	6
	Live	277	156	99	69	50	39	30	24	20	17	14	12	11	9	8	7
	Deflection (L/180)	500	382	195	113	71	47	33	24	18	14	11	8	7	5	4	4
	Deflection (L/240)	500	286	146	84	53	35	25	18	13	10	8	6	5	4	3	3
2 Span	Positive Wind	224	130	84	59	43	33	26	21	17	15	12	11	9	8	7	6
	Negative Wind	250	146	96	67	49	38	30	24	20	17	14	12	11	9	8	7
	Live	224	130	84	59	43	33	26	21	17	15	12	11	9	8	7	6
	Deflection (L/180)	500	500	384	222	139	93	65	48	36	27	21	17	14	11	9	8
	Deflection (L/240)	500	500	288	166	104	70	49	36	27	20	16	13	10	8	7	6
3 Span	Positive Wind	271	159	104	73	54	42	33	27	22	18	16	13	12	10	9	8
	Negative Wind	300	179	118	83	61	47	37	30	25	21	18	15	13	12	10	9
	Live	271	159	104	73	54	42	33	27	22	18	16	13	12	10	9	8
	Deflection (L/180)	500	500	300	174	109	73	51	37	28	21	17	13	11	9	7	6
	Deflection (L/240)	500	440	225	130	82	55	38	28	21	16	12	10	8	6	5	4
4 Span	Positive Wind	256	150	98	69	51	39	31	25	20	17	15	12	11	9	8	7
	Negative Wind	284	168	110	78	57	44	35	28	23	20	17	14	12	11	10	8
	Live	256	150	98	69	51	39	31	25	20	17	15	12	11	9	8	7
	Deflection (L/180)	500	500	319	184	116	77	54	39	29	23	18	14	11	9	8	6
	Deflection (L/240)	500	467	239	138	87	58	41	29	22	17	13	10	8	7	6	5

For SI: 1 inch = 25.4 mm; 1 foot = 305 mm; 1 ksi = 6.89 MPa; 1 psf = 48 Pa.

Notes:

1. Allowable uniform loads are based upon equal span lengths.
2. Live is the allowable live or snow load.
3. Deflection (L/180) is the allowable load that limits the panel's deflection to L/180 while under positive or live load.
4. Deflection (L/240) is the allowable load that limits the panel's deflection to L/240 while under positive or live load.
5. The weight of the panel has **NOT** been deducted from the allowable loads.
6. Positive Wind, Negative Wind, and Live Load values are limited to combined shear & bending using Eq. H2-1 of the AISI S100.
7. Positive Wind and Live Load values are limited by web crippling using a bearing length of 2 inches.
8. Web crippling values are determined using a ratio of the uniform load **actually** supported by the top flanges of the section.
9. Load Tables are limited to a maximum allowable load of 500 psf.
10. The deflection limits do not ensure against ponding. Roofs that deflect from snow loads or do not have sufficient slope or camber to ensure adequate drainage shall be investigated for ponding instability in accordance with Chapters 7 and 8 of ASCE 7.



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TABLE 17 — ALLOWABLE UNIFORM LOADS (PSF) FOR NO. 26 GAGE MULTI-RIB PANEL,  $F_y = 80$  ksi

Span Type	Load Type	Span in Feet															
		1.50	2.00	2.50	3.00	3.50	4.00	4.50	5.00	5.50	6.00	6.50	7.00	7.50	8.00	8.50	9.00
Single	Positive Wind	299	168	107	74	55	42	33	26	22	18	15	13	11	10	9	8
	Negative Wind	269	151	97	67	49	37	29	24	20	16	14	12	10	9	8	7
	Live	299	168	107	74	55	42	33	26	22	18	15	13	11	10	9	8
	Deflection (L/180)	500	349	179	103	65	43	30	22	16	12	10	8	6	5	4	3
	Deflection (L/240)	500	262	134	77	48	32	23	16	12	9	7	6	4	4	3	2
2 Span	Positive Wind	245	143	93	65	48	37	29	24	19	16	14	12	10	9	8	7
	Negative Wind	267	157	103	72	53	41	32	26	22	18	15	13	11	10	9	8
	Live	245	143	93	65	48	37	29	24	19	16	14	12	10	9	8	7
	Deflection (L/180)	500	500	350	202	127	85	60	43	32	25	19	15	12	10	8	7
	Deflection (L/240)	500	500	262	152	95	64	45	32	24	19	14	11	9	8	6	5
3 Span	Positive Wind	296	175	115	81	60	46	36	29	24	20	17	15	13	11	10	9
	Negative Wind	320	191	126	89	66	51	40	33	27	23	19	17	14	13	11	10
	Live	296	175	115	81	60	46	36	29	24	20	17	15	13	11	10	9
	Deflection (L/180)	500	500	274	158	100	67	47	34	25	19	15	12	10	8	6	5
	Deflection (L/240)	500	402	205	119	75	50	35	25	19	14	11	9	7	6	5	4
4 Span	Positive Wind	280	165	108	76	56	43	34	28	23	19	16	14	12	11	9	8
	Negative Wind	303	180	119	84	62	48	38	31	25	21	18	15	13	12	10	9
	Live	280	165	108	76	56	43	34	28	23	19	16	14	12	11	9	8
	Deflection (L/180)	500	500	291	168	106	71	49	36	27	21	16	13	10	8	7	6
	Deflection (L/240)	500	426	218	126	79	53	37	27	20	15	12	9	8	6	5	4

For SI: 1 inch = 2.54 mm; 1 foot = 305 mm; 1 ksi = 6.89 MPa; 1 psf = 48 Pa.

Notes:

1. Allowable uniform loads are based upon equal span lengths.
2. Live is the allowable live or snow load.
3. Deflection (L/180) is the allowable load that limits the panel's deflection to L/180 while under positive or live load.
4. Deflection (L/240) is the allowable load that limits the panel's deflection to L/240 while under positive or live load.
5. The weight of the panel has **NOT** been deducted from the allowable loads.
6. Positive Wind, Negative Wind, and Live Load values are limited to combined shear & bending using Eq. H2-1 of the AISI S100.
7. Positive Wind and Live Load values are limited by web crippling using a bearing length of 2 inches.
8. Web crippling values are determined using a ratio of the uniform load **actually** supported by the top flanges of the section.
9. Load Tables are limited to a maximum allowable load of 500 psf.
10. The deflection limits do not ensure against ponding. Roofs that deflect from snow loads or do not have sufficient slope or camber to ensure adequate drainage shall be investigated for ponding instability in accordance with Chapters 7 and 8 of ASCE 7.

TABLE 18—EFFECTIVE SECTION PROPERTIES FOR MULTI-RIB PB PANEL

SECTION PROPERTIES				TOP IN COMPRESSION				BOTTOM IN COMPRESSION			
GAGE	$F_y$ (KSI)	WEIGHT (PSF)	$V_a$ kip/ft.	$P_{a\_end}$ lbs/ft.	$P_{a\_int}$ lbs/ft.	$I_x$ (in. <sup>4</sup> /ft.)	$S_e$ (in. <sup>3</sup> /ft.)	$M_a$ kip-in./ft.	$I_x$ (in. <sup>4</sup> /ft.)	$S_e$ (in. <sup>3</sup> /ft.)	$M_a$ kip-in./ft.
24	50.0	1.13	0.9013	274.1	327.4	0.051	0.057	1.414	0.031	0.050	1.257
26	80.0	0.90	0.6910	211.0	244.0	0.034	0.036	1.092	0.022	0.035	1.046

For SI: 1 inch = 2.54 mm; 1 ksi = 6.89 MPa; 1 psf = 48 Pa.

Notes:

1. Section properties are calculated in accordance with the AISI S100-16.
2.  $V_a$  is the allowable shear.
3.  $P_a$  is the allowable load for web crippling on end and interior supports using a bearing length of 2 inches (5.08 mm).
4.  $I_x$  is for deflection determination.
5.  $S_e$  is for bending.
6.  $M_a$  is the allowable bending moment.
7. All values are for one foot of panel width.





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TABLE 19 — ALLOWABLE UNIFORM LOADS (PSF) FOR NO. 24 GAGE MULTI-RIB PB PANEL

Span Type	Load Type	Span in Feet															
		1.50	2.00	2.50	3.00	3.50	4.00	4.50	5.00	5.50	6.00	6.50	7.00	7.50	8.00	8.50	9.00
Single	Positive Wind	418	235	150	104	76	58	46	37	31	26	22	19	16	14	13	11
	Negative Wind	372	209	134	93	68	52	41	33	27	23	19	17	14	13	11	10
	Live	418	235	150	104	76	58	46	37	31	26	22	19	16	14	13	11
	Deflection (L/180)	500	500	287	166	104	70	49	35	26	20	16	13	10	8	7	6
	Deflection (L/240)	500	420	215	124	78	52	36	26	20	15	12	9	7	6	5	4
2 Span	Positive Wind	347	201	130	91	67	51	41	33	27	23	19	17	14	13	11	10
	Negative Wind	384	223	145	102	75	58	46	37	30	26	22	19	16	14	13	11
	Live	347	201	130	91	67	51	41	33	27	23	19	17	14	13	11	10
	Deflection (L/180)	500	500	500	321	202	135	95	69	52	40	31	25	20	16	14	11
	Deflection (L/240)	500	500	416	240	151	101	71	52	39	30	23	18	15	12	10	8
3 Span	Positive Wind	422	247	161	113	83	64	51	41	34	28	24	21	18	16	14	12
	Negative Wind	463	274	179	126	93	72	57	46	38	32	27	23	20	18	16	14
	Live	422	247	161	113	83	64	51	41	34	28	24	21	18	16	14	12
	Deflection (L/180)	500	500	434	251	158	106	74	54	40	31	24	19	16	13	11	9
	Deflection (L/240)	500	500	325	188	118	79	55	40	30	23	18	14	12	9	8	6
4 Span	Positive Wind	397	232	151	106	78	60	47	38	32	26	23	19	17	15	13	12
	Negative Wind	438	257	168	118	87	67	53	43	36	30	25	22	19	17	15	13
	Live	397	232	151	106	78	60	47	38	32	26	23	19	17	15	13	12
	Deflection (L/180)	500	500	461	266	168	112	79	57	43	33	26	21	17	14	11	9
	Deflection (L/240)	500	500	345	200	126	84	59	43	32	25	19	15	12	10	8	7

For SI: 1 inch = 2.54 mm; 1 foot = 305 mm; 1 ksi = 6.89 MPa; 1 psf = 48 Pa.

Notes:

1. Allowable uniform loads are based upon equal span lengths.
2. Live is the allowable live or snow load.
3. Deflection (L/180) is the allowable load that limits the panel's deflection to L/180 while under positive or live load.
4. Deflection (L/240) is the allowable load that limits the panel's deflection to L/240 while under positive or live load.
5. The weight of the panel has **NOT** been deducted from the allowable loads.
6. Positive Wind, Negative Wind, and Live Load values are limited to combined shear & bending using Eq. H2-1 of the AISI S100.
7. Positive Wind and Live Load values are limited by web crippling using a bearing length of 2 inches.
8. Web crippling values are determined using a ratio of the uniform load **actually** supported by the top flanges of the section.
9. Load Tables are limited to a maximum allowable load of 500 psf.
10. The deflection limits do not ensure against ponding. Roofs that deflect from snow loads or do not have sufficient slope or camber to ensure adequate drainage shall be investigated for ponding instability in accordance with Chapters 7 and 8 of ASCE 7.



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TABLE 20 — ALLOWABLE UNIFORM LOADS (PSF) FOR NO. 26 GAGE MULTI-RIB PB PANEL,  $F_y = 80$  ksi

		Span in Feet															
Span Type	Load Type	1.50	2.00	2.50	3.00	3.50	4.00	4.50	5.00	5.50	6.00	6.50	7.00	7.50	8.00	8.50	9.00
Single	Positive Wind	323	181	116	80	59	45	35	29	24	20	17	14	12	11	10	8
	Negative Wind	309	174	111	77	56	43	34	27	23	19	16	14	12	10	9	8
	Live	323	181	116	80	59	45	35	29	24	20	17	14	12	11	10	8
	Deflection (L/180)	500	375	192	111	69	46	32	24	18	13	10	8	7	5	4	4
	Deflection (L/240)	500	281	144	83	52	35	24	18	13	10	8	6	5	4	3	3
2 Span	Positive Wind	285	166	108	75	56	43	34	27	22	19	16	14	12	10	9	8
	Negative Wind	296	172	112	78	58	44	35	28	23	20	17	14	12	11	10	8
	Live	285	166	108	75	56	43	34	27	22	19	16	14	12	10	9	8
	Deflection (L/180)	500	500	381	220	139	93	65	47	35	27	21	17	14	11	9	8
	Deflection (L/240)	500	500	286	165	104	69	49	35	26	20	16	13	10	8	7	6
3 Span	Positive Wind	345	203	133	93	69	53	42	34	28	24	20	17	15	13	12	10
	Negative Wind	357	211	138	97	72	55	44	35	29	25	21	18	16	14	12	11
	Live	345	203	133	93	69	53	42	34	28	24	20	17	15	13	12	10
	Deflection (L/180)	500	500	299	173	108	73	51	37	28	21	17	13	11	9	7	6
	Deflection (L/240)	500	438	224	129	81	54	38	28	21	16	12	10	8	6	5	4
4 Span	Positive Wind	326	191	125	87	65	50	39	32	26	22	19	16	14	12	11	10
	Negative Wind	337	198	130	91	67	52	41	33	27	23	19	17	15	13	11	10
	Live	326	191	125	87	65	50	39	32	26	22	19	16	14	12	11	10
	Deflection (L/180)	500	500	317	183	115	77	54	39	29	22	18	14	11	9	8	6
	Deflection (L/240)	500	465	238	137	86	58	40	29	22	17	13	10	8	7	6	5

For SI: 1 inch = 25.4 mm; 1 foot = 305 mm; 1 ksi = 6.89 MPa; 1 psf = 48 Pa.

Notes:

1. Allowable uniform loads are based upon equal span lengths.
2. Live is the allowable live or snow load.
3. Deflection (L/180) is the allowable load that limits the panel's deflection to L/180 while under positive or live load.
4. Deflection (L/240) is the allowable load that limits the panel's deflection to L/240 while under positive or live load.
5. The weight of the panel has **NOT** been deducted from the allowable loads.
6. Positive Wind, Negative Wind, and Live Load values are limited to combined shear & bending using Eq. H2-1 of the AISI S100.
7. Positive Wind and Live Load values are limited by web crippling using a bearing length of 2 inches.
8. Web crippling values are determined using a ratio of the uniform load **actually** supported by the top flanges of the section.
9. Load Tables are limited to a maximum allowable load of 500 psf.
10. The deflection limits do not ensure against ponding. Roofs that deflect from snow loads or do not have sufficient slope or camber to ensure adequate drainage shall be investigated for ponding instability in accordance with Chapters 7 and 8 of ASCE 7.

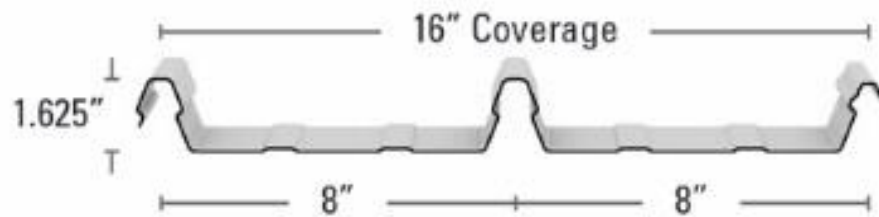


FIGURE 1—MIRAGE PANEL

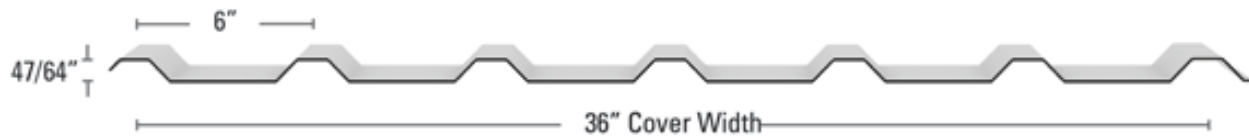


FIGURE 2—PBU-PANEL

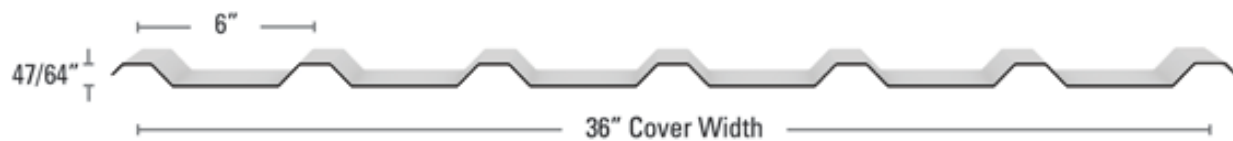


FIGURE 3—U PANEL

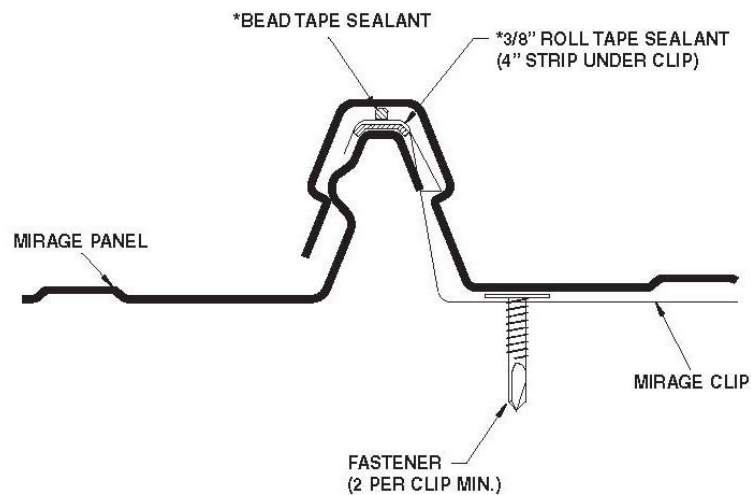


FIGURE 4 – MIRAGE SIDE LAP DETAIL

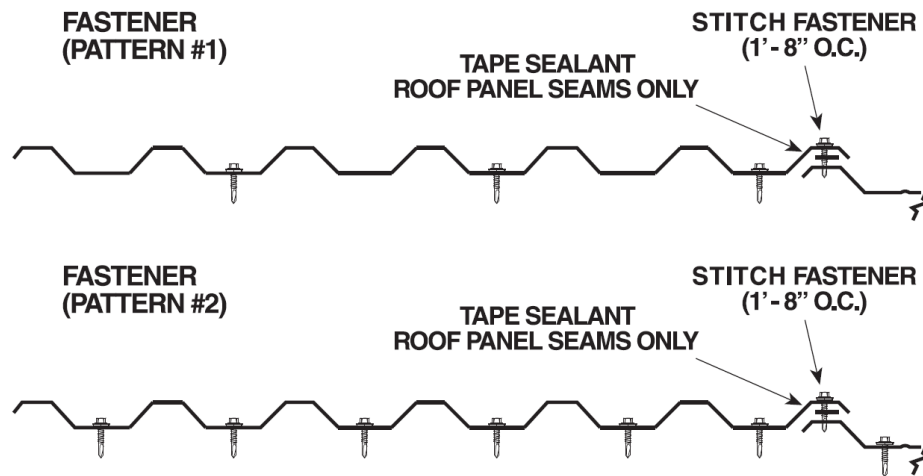


FIGURE 5 – TYPICAL FRAMING AND STITCH FASTENER PATTERNS

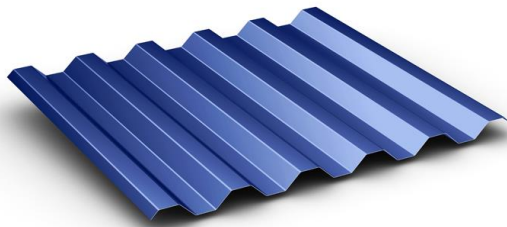


FIGURE 6—MEGA-RIB PANEL

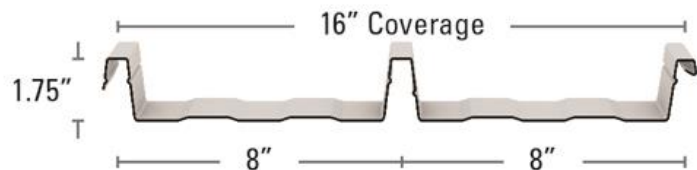
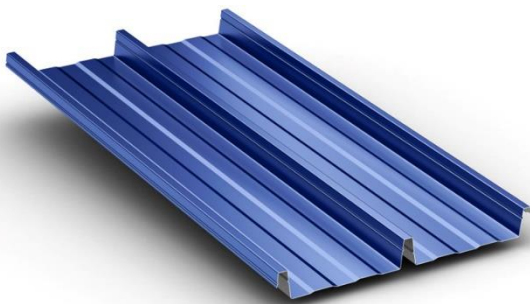


FIGURE 7—MIRAGE II PANEL



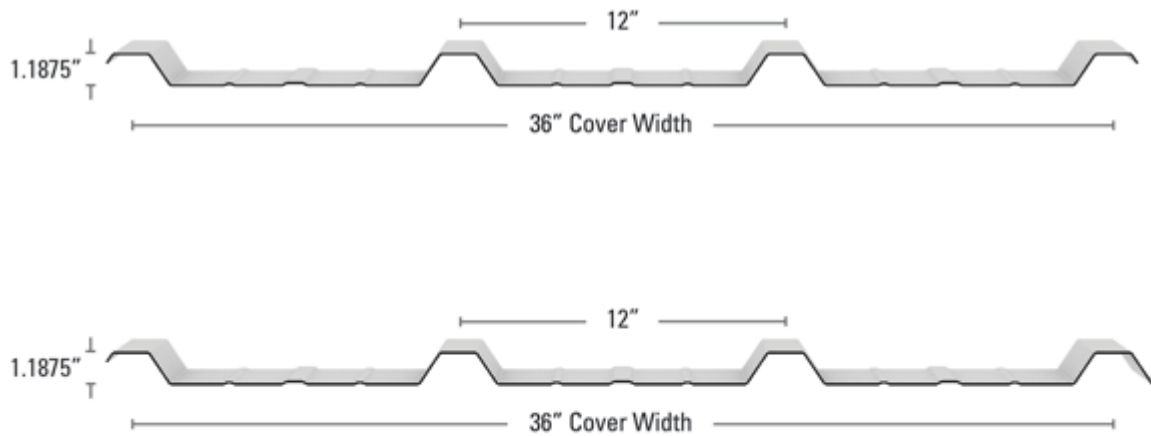


FIGURE 8—MULTI-RIB AND MULTI-RIB PB PANELS



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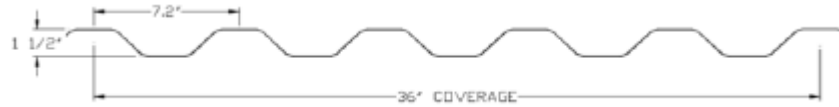
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TABLE 21 – ALLOWABLE (ASD) DIAPHRAGM STRENGTH AND STIFFNESS FOR MEGA-RIB NO. 24 GAGE,  $F_y = 50$  ksi



Stitch Screw & Edge Fastener Spacing = 12" o.c.									
1-Span			2-Span			3-Span			
Span (ft)	Strength (lb/ft) WindSeismic	Stiffness (kip/in)	Strength (lb/ft) WindSeismic	Stiffness (kip/in)	Strength (lb/ft) WindSeismic	Stiffness (kip/in)	Strength (lb/ft) WindSeismic	Stiffness (kip/in)	
3	391.1	340.1	30.67	335.7	291.9	4.45	314.6	273.6	7.00
4	352.8	306.8	33.91	306.0	266.1	5.74	288.9	251.2	8.87
5	327.0	284.3	35.96	286.9	249.5	6.95	272.5	237.0	10.55
6	308.5	268.2	37.32	273.6	237.9	8.09	261.3	227.2	12.07
7	294.7	256.2	38.26	263.9	229.4	9.15	253.1	220.1	13.45
8	283.9	246.9	38.92	256.4	223.0	10.14	246.8	214.6	14.70
Stitch Screw & Edge Fastener Spacing = 16" o.c.									
1-Span			2-Span			3-Span			
Span (ft)	Strength (lb/ft) WindSeismic	Stiffness (kip/in)	Strength (lb/ft) WindSeismic	Stiffness (kip/in)	Strength (lb/ft) WindSeismic	Stiffness (kip/in)	Strength (lb/ft) WindSeismic	Stiffness (kip/in)	
3	352.4	306.4	29.59	288.3	250.7	4.41	264.2	229.7	6.88
4	319.8	278.1	32.61	267.9	233.0	5.68	249.1	216.6	8.71
5	262.0	227.8	32.89	238.5	207.4	6.83	228.3	198.5	10.26
6	248.2	215.8	34.21	232.4	202.1	7.93	214.1	186.1	11.63
7	238.4	207.3	35.18	212.4	184.7	8.89	203.6	177.1	12.83
8	233.6	203.1	35.92	211.6	184.0	9.86	203.3	176.8	14.04
Stitch Screw & Edge Fastener Spacing = 20" o.c.									
1-Span			2-Span			3-Span			
Span (ft)	Strength (lb/ft) WindSeismic	Stiffness (kip/in)	Strength (lb/ft) WindSeismic	Stiffness (kip/in)	Strength (lb/ft) WindSeismic	Stiffness (kip/in)	Strength (lb/ft) WindSeismic	Stiffness (kip/in)	
3	307.4	267.3	28.19	262.1	227.9	4.38	245.9	213.8	6.83
4	282.6	245.7	31.02	225.3	195.9	5.60	220.5	191.8	8.56
5	262.0	227.8	32.89	219.3	190.7	6.77	204.7	178.0	10.06
6	213.0	185.2	32.27	197.7	171.9	7.79	180.9	157.3	11.25
7	208.2	181.0	33.32	182.3	158.5	8.71	173.6	151.0	12.41
8	180.8	157.2	32.17	172.0	149.6	9.55	169.1	147.0	13.44

For SI: 1 inch = 25.4 mm, 1 foot = 305 mm, 1 ksi = 1 MPa, 1 lb/ft = 14.59 N/m, 1 kip/in = 1751 N/m

Notes:

1. Capacities are calculated in accordance with AISI S310-16.
2. Structural Fastener to Supports: #12-14, 14.4 inches on center spacing across interior supports, 7.2 inches on center spacing across end supports.
3. Fastener spacing at edge panels parallel to deck ribs: #12-14 spaced as indicated in table.
4. Stitch Fastener: #1/4-14 Laptek spaced as indicated in table.
5. Structural Fasteners shall be located at the panel edge at each support and spaced as indicated in table.
6. Minimum 0.059 inch thick steel supports
7. Panels are not subject to uplift.
8. Insulation under the panel is outside the scope of this table.
9. Safety Factors for ASD are in accordance with AISI S310-16 Table B1.1 for wind and seismic.



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TABLE 22 – ALLOWABLE (ASD) DIAPHRAGM STRENGTH AND STIFFNESS FOR MEGA-RIB NO. 26 GAGE,  $F_y = 80$  ksi



**Stitch Screw & Edge Fastener Spacing = 12" o.c.**

1-Span			2-Span			3-Span			
Span (ft)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)
	Wind	Seismic		Wind	Seismic		Wind	Seismic	
3	301.4	262.1	20.04	255.8	222.5	2.50	238.6	207.5	4.01
4	269.4	234.2	23.03	231.1	201.0	3.26	217.2	188.9	5.16
5	247.9	215.6	25.16	215.3	187.2	3.99	203.7	177.2	6.23
6	230.8	200.7	26.72	204.4	177.7	4.69	194.4	169.1	7.23
7	217.2	188.9	27.90	196.4	170.7	5.35	187.7	163.2	8.16
8	208.9	181.6	28.82	190.2	165.4	5.99	182.5	158.7	9.04

**Stitch Screw & Edge Fastener Spacing = 16" o.c.**

1-Span			2-Span			3-Span			
Span (ft)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)
	Wind	Seismic		Wind	Seismic		Wind	Seismic	
3	272.0	236.5	19.52	220.4	191.6	2.49	201.1	174.9	3.96
4	244.5	212.6	22.35	202.7	176.3	3.24	187.7	163.2	5.10
5	192.2	167.2	23.43	176.5	153.5	3.94	171.0	148.7	6.12
6	180.5	157.0	24.89	169.8	147.7	4.63	157.9	137.3	7.05
7	174.1	151.4	26.03	155.3	135.0	5.25	149.0	129.6	7.91
8	171.2	148.9	26.94	154.7	134.5	5.88	149.2	129.7	8.75

**Stitch Screw & Edge Fastener Spacing = 20" o.c.**

1-Span			2-Span			3-Span			
Span (ft)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)
	Wind	Seismic		Wind	Seismic		Wind	Seismic	
3	238.6	207.5	18.82	201.1	174.9	2.48	187.7	163.2	3.95
4	209.8	182.5	21.50	167.7	145.9	3.21	166.3	144.6	5.04
5	192.2	167.2	23.43	161.5	140.4	3.92	151.2	131.5	6.04
6	155.4	135.1	23.72	144.7	125.8	4.57	132.8	115.4	6.89
7	152.6	132.7	24.87	133.7	116.3	5.18	127.4	110.8	7.72
8	133.5	116.1	24.52	126.4	109.9	5.75	124.1	107.9	8.48

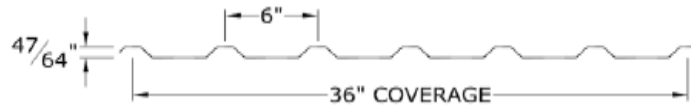
For SI: 1 inch = 25.4 mm, 1 foot = 305 mm, 1 ksi = 1 MPa, 1 lb/ft = 14.59 N/m, 1 kip/in = 1751 N/m

**Notes:**

- Capacities are calculated in accordance with AISI S310-16, North American Standard for the Design of Profiled Steel Diaphragm Panels.
- Structural Fastener to Supports: #12-14, 14.4 inches on center spacing across interior supports, 7.2 inches on center spacing across end supports.
- Fastener spacing at edge panels parallel to deck ribs: #12-14 spaced as indicated in table.
- Stitch Fastener: #1/4-14 Laptek spaced as indicated in table.
- Structural Fasteners shall be located at the panel edge at each support.
- Minimum 0.059-inch-Thick Steel Supports.
- Panels are not subject to uplift.
- Insulation under the panel is outside the scope of this table.
- Safety Factors for ASD are in accordance with AISI S310-16 Table B1.1 for wind and seismic.



TABLE 23 – ALLOWABLE (ASD) DIAPHRAGM STRENGTH AND STIFFNESS FOR U-PANEL No. 24 GAGE,  $F_y = 50$  ksi



**Stitch Screw & Edge Fastener Spacing = 12" o.c.**

Span (ft)	1-Span		Stiffness (kip/in)	2-Span		Stiffness (kip/in)	3-Span		Stiffness (kip/in)
	Strength (lb/ft) Wind	Seismic		Strength (lb/ft) Wind	Seismic		Strength (lb/ft) Wind	Seismic	
3	440.9	383.4	41.88	359.1	312.2	11.16	327.70	285.0	16.21
4	392.2	341.0	43.71	324.0	281.7	13.77	298.90	259.9	19.26
5	359.6	312.7	44.48	301.7	262.3	15.97	280.90	244.3	21.65
6	336.4	292.5	44.76	286.3	249.0	17.84	268.70	233.6	23.58
7	305.6	277.5	44.81	275.1	239.2	19.46	259.70	225.9	25.15
8	234.0	234.0	44.74	234.0	231.8	20.87	234.00	220.0	26.46

**Stitch Screw & Edge Fastener Spacing = 16" o.c.**

Span (ft)	1-Span		Stiffness (kip/in)	2-Span		Stiffness (kip/in)	3-Span		Stiffness (kip/in)
	Strength (lb/ft) Wind	Seismic		Strength (lb/ft) Wind	Seismic		Strength (lb/ft) Wind	Seismic	
3	398.8	346.8	40.26	307.1	267.0	10.95	272.4	236.9	15.64
4	356.5	310.0	41.92	282.7	245.8	13.46	255.8	222.5	18.54
5	296.6	257.9	40.54	249.9	217.3	15.37	221.1	192.2	20.52
6	282.0	245.2	40.93	219.4	190.8	17.16	184.2	160.2	22.02
7	271.3	235.9	41.14	188.1	163.5	18.42	157.9	137.3	23.18
8	243.8	212.0	41.25	164.6	143.1	19.77	138.2	120.1	24.49

**Stitch Screw & Edge Fastener Spacing = 20" o.c.**

Span (ft)	1-Span		Stiffness (kip/in)	2-Span		Stiffness (kip/in)	3-Span		Stiffness (kip/in)
	Strength (lb/ft) Wind	Seismic		Strength (lb/ft) Wind	Seismic		Strength (lb/ft) Wind	Seismic	
3	351.4	305.6	38.24	279.1	242.7	10.81	252.8	219.8	15.39
4	317.8	276.4	39.79	238.5	207.4	13.05	225.6	196.1	17.93
5	296.6	257.9	40.54	231.7	201.5	15.11	208.8	181.5	19.80
6	251.3	218.5	38.61	211.6	184.0	16.56	186.7	162.3	20.80
7	243.3	211.5	38.98	195.5	170.0	17.73	179.6	156.2	21.90
8	210.9	183.4	36.99	183.3	159.4	18.67	174.1	151.4	22.80

For SI: 1 inch = 25.4 mm, 1 foot = 305 mm, 1 ksi = 1 MPa, 1 lb/ft = 14.59 N/m, 1 kip/in = 1751 N/m

**Notes:**

1. Capacities are calculated in accordance with AISI S310-16, North American Standard for the Design of Profiled Steel Diaphragm Panels.
2. Structural Fastener to Supports: #12-14, 12 inches on center spacing across interior supports, 6 inches on center spacing across end supports.
3. Fastener spacing at edge panels parallel to deck ribs: #12-14 spaced as indicated in table.
4. Stitch Fastener: #1/4-14 Laptek spaced as indicated in table.
5. Structural Fasteners shall be located at the panel edge at each support.
6. Minimum 0.059-inch-Thick Steel Supports.
7. Panels are not subject to uplift.
8. Insulation under the panel is outside the scope of this table.
9. Safety Factors for ASD are in accordance with AISI S310-16 Table B1.1 for wind and seismic.





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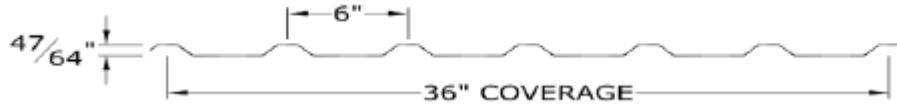
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TABLE 24 – ALLOWABLE (ASD) DIAPHRAGM STRENGTH AND STIFFNESS FOR U-PANEL No. 26 GAGE,  $F_y = 50$  ksi



Stitch Screw & Edge Fastener Spacing = 12" o.c.									
1-Span			2-Span			3-Span			
Span (ft)	Strength (lb/ft) Wind	Strength (lb/ft) Seismic	Stiffness (kip/in)	Strength (lb/ft) Wind	Strength (lb/ft) Seismic	Stiffness (kip/in)	Strength (lb/ft) Wind	Strength (lb/ft) Seismic	Stiffness (kip/in)
3	331.9	288.6	29.30	266.4	231.6	6.54	241.50	210.0	9.88
4	292.5	254.4	31.70	238.3	207.2	8.26	218.50	190.0	12.10
5	266.3	231.6	33.09	220.5	191.7	9.78	204.10	177.5	13.97
6	247.8	215.5	33.93	208.3	181.1	11.14	194.40	169.0	15.55
7	223.3	203.5	34.45	199.4	173.4	12.36	187.30	162.8	16.90
8	171.0	171.0	34.78	171.0	167.5	13.46	171.00	158.2	18.07

Stitch Screw & Edge Fastener Spacing = 16" o.c.									
1-Span			2-Span			3-Span			
Span (ft)	Strength (lb/ft) Wind	Strength (lb/ft) Seismic	Stiffness (kip/in)	Strength (lb/ft) Wind	Strength (lb/ft) Seismic	Stiffness (kip/in)	Strength (lb/ft) Wind	Strength (lb/ft) Seismic	Stiffness (kip/in)
3	301.2	261.9	28.40	228.9	199.0	6.46	201.8	175.5	9.64
4	266.6	231.8	30.63	208.6	181.4	8.13	187.6	163.1	11.78
5	221.0	192.1	30.60	183.4	159.5	9.52	170.3	148.1	13.43
6	207.8	180.7	31.41	176.8	153.8	10.83	158.5	137.8	14.77
7	197.1	171.4	31.97	161.7	140.6	11.88	149.9	130.4	15.88
8	171.0	164.4	32.37	159.0	138.3	12.93	149.0	129.6	17.02

Stitch Screw & Edge Fastener Spacing = 20" o.c.									
1-Span			2-Span			3-Span			
Span (ft)	Strength (lb/ft) Wind	Strength (lb/ft) Seismic	Stiffness (kip/in)	Strength (lb/ft) Wind	Strength (lb/ft) Seismic	Stiffness (kip/in)	Strength (lb/ft) Wind	Strength (lb/ft) Seismic	Stiffness (kip/in)
3	267.1	232.2	27.25	208.9	181.7	6.40	187.9	163.4	9.53
4	238.8	207.7	29.34	177.2	154.1	7.96	166.1	144.4	11.50
5	221.0	192.1	30.60	170.4	148.2	9.41	152.7	132.8	13.08
6	183.3	159.4	29.86	153.1	133.1	10.56	134.8	117.3	14.14
7	176.1	153.1	30.49	140.7	122.4	11.55	128.9	112.1	15.19
8	152.3	132.4	29.38	131.4	114.3	12.39	124.5	108.2	16.08

For SI: 1 inch = 25.4 mm, 1 foot = 305 mm, 1 ksi = 1 MPa, 1 lb/ft = 14.59 N/m, 1 kip/in = 1751 N/m

Notes:

1. Capacities are calculated in accordance with AISI S310-16, North American Standard for the Design of Profiled Steel Diaphragm Panels.
2. Structural Fastener to Supports: #12-14, 12 inches on center spacing across interior supports, 6 inches on center spacing across end supports.
3. Fastener spacing at edge panels parallel to deck ribs: #12-14 spaced as indicated in table.
4. Stitch Fastener: #1/4-14 Laptek spaced as indicated in table.
5. Structural Fasteners shall be located at the panel edge at each support.
6. Minimum 0.059-inch-Thick Steel Supports.
7. Panels are not subject to uplift.
8. Insulation under the panel is outside the scope of this table.
9. Safety Factors for ASD are in accordance with AISI S310-16 Table B1.1 for wind and seismic.



# EVALUATION REPORT

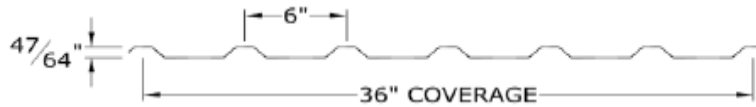
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TABLE 25 – ALLOWABLE (ASD) DIAPHRAGM STRENGTH AND STIFFNESS FOR U-PANEL No. 26 GAGE,  $F_y = 80$  ksi



Stitch Screw & Edge Fastener Spacing = 12" o.c.									
1-Span			2-Span			3-Span			
Span (ft)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)
	Wind	Seismic		Wind	Seismic		Wind	Seismic	
3	328.9	286.0	27.99	263.5	229.1	6.13	238.7	207.6	9.29
4	289.5	251.8	30.43	235.4	204.7	7.75	215.7	187.6	11.42
5	263.4	229.0	31.86	217.7	189.3	9.20	201.4	175.2	13.22
6	244.9	213.0	32.75	205.5	178.7	10.50	191.7	166.7	14.75
7	213.6	201.0	33.31	196.6	171.0	11.67	184.6	160.5	16.07
8	163.5	163.5	33.68	163.5	163.5	12.73	163.5	155.9	17.22

Stitch Screw & Edge Fastener Spacing = 16" o.c.									
1-Span			2-Span			3-Span			
Span (ft)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)
	Wind	Seismic		Wind	Seismic		Wind	Seismic	
3	298.5	259.6	27.16	226.6	197.0	6.06	199.6	173.6	9.08
4	264.0	229.6	29.43	206.2	179.3	7.64	185.3	161.2	11.13
5	218.7	190.2	29.51	181.2	157.5	8.97	168.1	146.2	12.73
6	205.0	178.2	30.37	174.6	151.8	10.23	156.4	136.0	14.04
7	194.3	168.9	30.96	159.3	138.5	11.24	147.7	128.4	15.13
8	163.5	162.0	31.39	156.6	136.1	12.26	146.7	127.5	16.24

Stitch Screw & Edge Fastener Spacing = 20" o.c.									
1-Span			2-Span			3-Span			
Span (ft)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)
	Wind	Seismic		Wind	Seismic		Wind	Seismic	
3	265.0	230.4	26.10	206.9	179.9	6.01	185.9	161.7	8.98
4	236.6	205.8	28.21	175.3	152.4	7.49	164.2	142.8	10.87
5	218.7	190.2	29.51	168.1	146.2	8.87	150.8	131.1	12.41
6	180.9	157.3	28.90	151.0	131.3	9.98	132.9	115.6	13.46
7	173.7	151.0	29.55	138.7	120.6	10.94	127.0	110.5	14.50
8	150.2	130.6	28.53	129.5	112.6	11.76	122.6	106.6	15.38

For SI: 1 inch = 25.4 mm, 1 foot = 305 mm, 1 ksi = 1 MPa, 1 lb/ft = 14.59 N/m, 1 kip/in = 1751 N/m

Notes:

1. Capacities are calculated in accordance with AISI S310-16, North American Standard for the Design of Profiled Steel Diaphragm Panels.
2. Structural Fastener to Supports: #12-14, 12 inches on center spacing across interior supports, 6 inches on center spacing across end supports.
3. Fastener spacing at edge panels parallel to deck ribs: #12-14 spaced as indicated in table.
4. Stitch Fastener: #1/4-14 Laptek spaced as indicated in table.
5. Structural Fasteners shall be located at the panel edge at each support.
6. Minimum 0.059-inch-Thick Steel Supports.
7. Panels are not subject to uplift.
8. Insulation under the panel is outside the scope of this table.
9. Safety Factors for ASD are in accordance with AISI S310-16 Table B1.1 for wind and seismic.



# EVALUATION REPORT

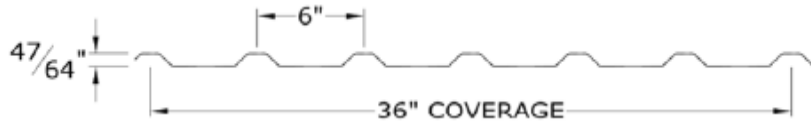
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TABLE 26 – ALLOWABLE (ASD) DIAPHRAGM STRENGTH AND STIFFNESS FOR U-PANEL PB, No. 24 GAGE, Fy =50 ksi



Stitch Screw & Edge Fastener Spacing = 12" o.c.									
Span (ft)	1-Span		Stiffness (kip/in)	2-Span		Stiffness (kip/in)	3-Span		Stiffness (kip/in)
	Strength (lb/ft)	Seismic		Strength (lb/ft)	Seismic		Strength (lb/ft)	Seismic	
3	440.9	383.4	41.88	359.1	312.2	11.16	327.70	285.0	16.21
4	392.2	341.0	43.71	324.0	281.7	13.77	298.90	259.9	19.26
5	359.6	312.7	44.48	301.7	262.3	15.97	280.90	244.3	21.65
6	336.4	292.5	44.76	286.3	249.0	17.84	268.70	233.6	23.58
7	319.1	277.5	44.81	275.1	239.2	19.46	259.70	225.9	25.15
8	255.6	255.6	44.74	255.6	231.8	20.87	253.00	220.0	26.46

Stitch Screw & Edge Fastener Spacing = 16" o.c.									
Span (ft)	1-Span		Stiffness (kip/in)	2-Span		Stiffness (kip/in)	3-Span		Stiffness (kip/in)
	Strength (lb/ft)	Seismic		Strength (lb/ft)	Seismic		Strength (lb/ft)	Seismic	
3	398.8	346.8	40.26	307.1	267.0	10.95	272.4	236.9	15.64
4	356.5	310.0	41.92	282.7	245.8	13.46	255.8	222.5	18.54
5	296.6	257.9	40.54	249.9	217.3	15.37	233.6	203.1	20.52
6	282.0	245.2	40.93	242.4	210.8	17.16	218.4	189.9	22.02
7	271.3	235.9	41.14	223.9	194.7	18.42	207.4	180.3	23.18
8	255.6	228.9	41.25	221.4	192.5	19.77	206.9	179.9	24.49

Stitch Screw & Edge Fastener Spacing = 20" o.c.									
Span (ft)	1-Span		Stiffness (kip/in)	2-Span		Stiffness (kip/in)	3-Span		Stiffness (kip/in)
	Strength (lb/ft)	Seismic		Strength (lb/ft)	Seismic		Strength (lb/ft)	Seismic	
3	351.4	305.6	38.24	279.1	242.7	10.81	252.8	219.8	15.39
4	317.8	276.4	39.79	238.5	207.4	13.05	225.6	196.1	17.93
5	296.6	257.9	40.54	231.7	201.5	15.11	208.8	181.5	19.80
6	251.3	218.5	38.61	211.6	184.0	16.56	186.7	162.3	20.80
7	243.3	211.5	38.98	195.5	170.0	17.73	179.6	156.2	21.90
8	210.9	183.4	36.99	183.3	159.4	18.67	174.1	151.4	22.80

For SI: 1 inch =25.4 mm, 1 foot =305 mm, 1 ksi = 1 MPa, 1 lb/ft = 14.59 N/m, 1 kip/in = 1751 N/m

#### Notes:

1. Capacities are calculated in accordance with AISI S310-16, North American Standard for the Design of Profiled Steel Diaphragm Panels.
2. Structural Fastener to Supports: #12-14, 12 inches on center spacing across interior supports, 6 inches on center spacing across end supports.
3. Fastener spacing at edge panels parallel to deck ribs: #12-14 spaced as indicated in table.
4. Stitch Fastener: #1/4-14 Laptek spaced as indicated in table.
5. Structural Fasteners shall be located at the panel edge at each support.
6. Minimum 0.059-inch-Thick Steel Supports.
7. Panels are not subject to uplift.
8. Insulation under the panel is outside the scope of this table.
9. Safety Factors for ASD are in accordance with AISI S310-16 Table B1.1 for wind and seismic.



# EVALUATION REPORT

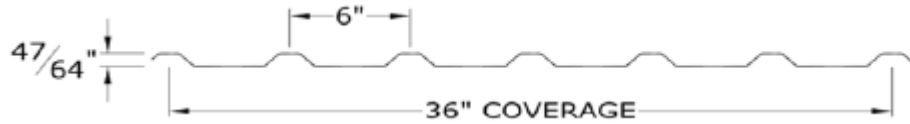
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TABLE 27 – ALLOWABLE (ASD) DIAPHRAGM STRENGTH AND STIFFNESS FOR U-PANEL PB, No. 26 GAGE, Fy =80 ksi



Stitch Screw & Edge Fastener Spacing = 12" o.c.									
Span (ft)	1-Span		Stiffness (kip/in)	2-Span		Stiffness (kip/in)	3-Span		Stiffness (kip/in)
	Strength (lb/ft) Wind	Seismic		Strength (lb/ft) Wind	Seismic		Strength (lb/ft) Wind	Seismic	
3	340.2	295.8	29.30	273.0	237.4	6.54	247.60	215.3	9.88
4	299.8	260.7	31.70	244.2	212.4	8.26	224.00	194.7	12.10
5	273.0	237.4	33.09	226.0	196.5	9.78	209.20	182.0	13.97
6	254.0	220.9	33.93	213.5	185.6	11.14	199.20	173.2	15.55
7	223.3	208.6	34.45	204.3	177.7	12.36	192.00	166.9	16.90

Stitch Screw & Edge Fastener Spacing = 16" o.c.									
Span (ft)	1-Span		Stiffness (kip/in)	2-Span		Stiffness (kip/in)	3-Span		Stiffness (kip/in)
	Strength (lb/ft) Wind	Seismic		Strength (lb/ft) Wind	Seismic		Strength (lb/ft) Wind	Seismic	
3	308.7	268.4	28.40	234.6	204.0	6.46	206.9	179.9	9.64
4	273.3	237.6	30.63	213.8	185.9	8.13	192.3	167.2	11.78
5	226.5	196.9	30.60	188.0	163.4	9.52	174.6	151.8	13.43
6	213.0	185.2	31.41	181.2	157.6	10.83	162.5	141.3	14.77
7	202.0	175.7	31.97	165.8	144.1	11.88	153.7	133.6	15.88
8	171.0	168.5	32.37	163.0	141.7	12.93	152.7	132.8	17.02

Stitch Screw & Edge Fastener Spacing = 20" o.c.									
Span (ft)	1-Span		Stiffness (kip/in)	2-Span		Stiffness (kip/in)	3-Span		Stiffness (kip/in)
	Strength (lb/ft) Wind	Seismic		Strength (lb/ft) Wind	Seismic		Strength (lb/ft) Wind	Seismic	
3	273.8	238.1	27.25	214.1	186.2	6.40	192.6	167.5	9.53
4	244.8	212.9	29.34	181.6	157.9	7.96	170.3	148.1	11.50
5	226.5	196.9	30.60	174.7	151.9	9.41	156.5	136.1	13.08
6	187.8	163.3	29.86	156.9	136.4	10.56	138.2	120.2	14.14
7	180.5	156.9	30.49	144.2	125.4	11.55	132.1	114.9	15.19
8	156.1	135.7	29.38	134.7	117.1	12.39	127.6	110.9	16.08

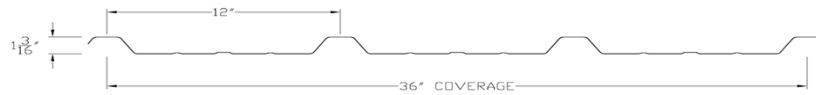
For SI: 1 inch =25.4 mm, 1 foot =305 mm, 1 ksi = 1 MPa, 1 lb/ft = 14.59 N/m, 1 kip/in = 1751 N/m

Notes:

1. Capacities are calculated in accordance with AISI S310-16, North American Standard for the Design of Profiled Steel Diaphragm Panels.
2. Structural Fastener to Supports: #12-14, 12 inches on center spacing across interior supports, 6 inches on center spacing across end supports.
3. Fastener spacing at edge panels parallel to deck ribs: #12-14 spaced as indicated in table.
4. Stitch Fastener: #1/4-14 Laptek spaced as indicated in table.
5. Structural Fasteners shall be located at the panel edge at each support.
6. Minimum 0.059-inch-Thick Steel Supports.
7. Panels are not subject to uplift.
8. Insulation under the panel is outside the scope of this table.
9. Safety Factors for ASD are in accordance with AISI S310-16 Table B1.1 for wind and seismic.



TABLE 28 – ALLOWABLE (ASD) DIAPHRAGM STRENGTH AND STIFFNESS FOR MULTI-RIB, No. 24 GAGE, Fy =50 ksi



Stitch Screw & Edge Fastener Spacing = 12" o.c.									
1-Span			2-Span			3-Span			
Span (ft)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)
	Wind	Seismic		Wind	Seismic		Wind	Seismic	
3	445.3	387.2	18.36	362.5	315.2	26.83	330.6	287.5	32.71
4	395.9	344.3	21.75	326.7	284.1	29.79	301.2	261.9	34.79
5	362.8	315.5	24.34	304.0	264.3	31.74	282.8	246.0	36.02
6	339.3	295.0	26.35	288.3	250.7	33.10	270.3	235.0	36.79
7	321.7	279.7	27.94	276.8	240.7	34.09	261.1	227.1	37.32
8	308.1	267.9	29.22	268.1	233.1	34.83	254.2	221.1	37.69

Stitch Screw & Edge Fastener Spacing = 16" o.c.									
1-Span			2-Span			3-Span			
Span (ft)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)
	Wind	Seismic		Wind	Seismic		Wind	Seismic	
3	403.8	351.1	18.05	310.9	270.3	25.64	275.6	239.7	30.50
4	360.6	313.6	21.31	285.7	248.4	28.42	258.3	224.6	32.54
5	300.3	261.1	23.13	252.4	219.5	29.48	235.6	204.9	33.03
6	281.4	244.7	24.99	244.5	212.6	30.84	220.1	191.4	33.18
7	267.8	232.8	26.49	223.3	194.2	31.03	208.5	181.3	33.18
8	257.6	224.0	27.71	220.2	191.5	31.89	207.8	180.7	33.84

Stitch Screw & Edge Fastener Spacing = 20" o.c.									
1-Span			2-Span			3-Span			
Span (ft)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)
	Wind	Seismic		Wind	Seismic		Wind	Seismic	
3	357.0	310.4	17.64	283.0	246.1	24.90	256.1	222.7	29.59
4	322.3	280.3	20.75	241.7	210.2	26.68	228.1	198.4	30.72
5	300.3	261.1	23.13	233.1	202.7	28.56	210.7	183.2	31.22
6	246.2	214.1	24.12	209.8	182.4	28.99	185.9	161.7	30.50
7	237.6	206.6	25.59	193.1	167.9	29.14	178.3	155.0	30.64
8	204.8	178.1	25.75	180.6	157.1	29.14	172.6	150.0	30.72

For SI: 1 inch =25.4 mm, 1 foot =305 mm, 1 ksi = 1 MPa, 1 lb/ft = 14.59 N/m, 1 kip/in = 1751 N/m

Notes:

1. Capacities are calculated in accordance with AISI S310-16, North American Standard for the Design of Profiled Steel Diaphragm Panels.
2. Structural Fastener to Supports: #12-14, 12 inches on center spacing across interior supports, 6 inches on center spacing across end supports.
3. Fastener spacing at edge panels parallel to deck ribs: #12-14 spaced as indicated in the table.
4. Stitch Fastener: #1/4-14 Laptek spaced as indicated in the table.
5. Structural Fasteners shall be located at the panel edge at each support.
6. Minimum 0.059-inch-Thick Steel Supports.
7. Panels are not subject to uplift.
8. Insulation under the panel is outside the scope of this table.
9. Safety Factors for ASD are in accordance with AISI S310-16 Table B1.1 for wind and seismic.



# EVALUATION REPORT

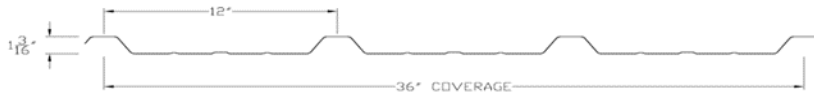
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TABLE 29 – ALLOWABLE (ASD) DIAPHRAGM STRENGTH AND STIFFNESS FOR MULTI-RIB, No. 26 GAGE, Fy =50 ksi



Stitch Screw & Edge Fastener Spacing = 12" o.c.									
1-Span			2-Span			3-Span			
Span (ft)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)
	Wind	Seismic		Wind	Seismic		Wind	Seismic	
3	335.5	291.8	11.15	269.2	234.1	17.58	243.9	212.1	22.74
4	295.6	257.1	13.62	240.5	209.1	20.26	220.4	191.6	25.06
5	269.0	233.9	15.66	222.4	193.4	22.21	205.7	178.9	26.60
6	250.2	217.5	17.34	209.9	182.5	23.68	195.7	170.1	27.68
7	233.7	203.3	18.76	200.7	174.6	24.82	188.4	163.8	28.47
8	220.1	191.4	19.96	193.2	168.0	25.73	182.9	159.0	29.08

Stitch Screw & Edge Fastener Spacing = 16" o.c.									
1-Span			2-Span			3-Span			
Span (ft)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)
	Wind	Seismic		Wind	Seismic		Wind	Seismic	
3	305.2	265.4	11.02	232.0	201.7	17.00	204.4	177.7	21.52
4	270.0	234.8	13.43	211.0	183.5	19.54	189.6	164.9	23.73
5	218.4	189.9	15.08	183.4	159.5	20.94	171.7	149.3	24.74
6	202.8	176.4	16.67	175.5	152.6	22.36	158.2	137.6	25.34
7	191.7	166.7	18.01	159.4	138.6	22.97	148.6	129.2	25.71
8	183.4	159.5	19.15	156.4	136.0	23.90	147.5	128.2	26.46

Stitch Screw & Edge Fastener Spacing = 20" o.c.									
1-Span			2-Span			3-Span			
Span (ft)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)
	Wind	Seismic		Wind	Seismic		Wind	Seismic	
3	271.5	236.1	10.85	212.1	184.4	16.62	190.5	165.7	21.01
4	241.8	210.3	13.17	176.9	153.8	18.59	167.5	145.6	22.63
5	218.4	189.9	15.08	168.7	146.7	20.41	152.1	132.3	23.58
6	178.3	155.1	16.23	151.0	131.3	21.25	133.7	116.3	23.55
7	170.7	148.4	17.54	138.4	120.3	21.79	127.6	110.9	23.98
8	146.6	127.5	18.08	128.9	112.1	22.13	123.0	106.9	24.28

For SI: 1 inch =25.4 mm, 1 foot =305 mm, 1 ksi = 1 MPa, 1 lb/ft = 14.59 N/m, 1 kip/in = 1751 N/m

#### Notes:

1. Capacities are calculated in accordance with AISI S310-16, North American Standard for the Design of Profiled Steel Diaphragm Panels.
2. Structural Fastener to Supports: #12-14, 12 inches on center spacing across interior supports, 6 inches on center spacing across end supports.
3. Fastener spacing at edge panels parallel to deck ribs: #12-14 spaced as indicated in table.
4. Stitch Fastener: #1/4-14 Laptek spaced as indicated in table.
5. Structural Fasteners shall be located at the panel edge at each support.
6. Minimum 0.059-inch-Thick Steel Supports.
7. Panels are not subject to uplift.
8. Insulation under the panel is outside the scope of this table.
9. Safety Factors for ASD are in accordance with AISI S310-16 Table B1.1 for wind and seismic.





# EVALUATION REPORT

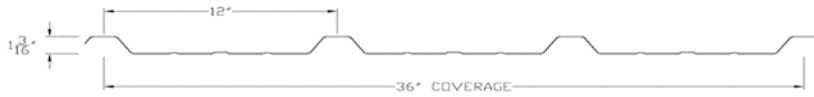
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TABLE 30 – ALLOWABLE (ASD) DIAPHRAGM STRENGTH AND STIFFNESS FOR MULTI-RIB, No. 26 GAGE, Fy =80 ksi



Stitch Screw & Edge Fastener Spacing = 12" o.c.									
1-Span			2-Span			3-Span			
Span (ft)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)
	Wind	Seismic		Wind	Seismic		Wind	Seismic	
3	332.5	289.1	10.48	266.3	231.6	16.66	241.1	209.6	21.71
4	292.7	254.5	12.85	237.7	206.7	19.29	217.6	189.2	24.03
5	266.1	231.4	14.81	219.6	190.9	21.22	203.0	176.5	25.59
6	247.3	215.0	16.45	207.1	180.1	22.69	193.0	167.8	26.69
7	230.1	200.1	17.83	198.0	172.2	23.83	185.7	161.5	27.50
8	216.6	188.3	19.01	190.0	165.3	24.75	180.2	156.7	28.13

Stitch Screw & Edge Fastener Spacing = 16" o.c.									
1-Span			2-Span			3-Span			
Span (ft)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)
	Wind	Seismic		Wind	Seismic		Wind	Seismic	
3	302.6	263.1	10.37	229.6	199.7	16.13	202.2	175.8	20.57
4	267.3	232.5	12.67	208.6	181.4	18.62	187.3	162.9	22.79
5	215.5	187.4	14.29	180.8	157.2	20.04	169.3	147.2	23.83
6	199.9	173.9	15.84	172.9	150.3	21.45	155.8	135.5	24.47
7	188.8	164.2	17.15	156.9	136.4	22.09	146.3	127.2	24.88
8	180.5	156.9	18.27	153.9	133.9	23.02	145.1	126.2	25.64

Stitch Screw & Edge Fastener Spacing = 20" o.c.									
1-Span			2-Span			3-Span			
Span (ft)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)
	Wind	Seismic		Wind	Seismic		Wind	Seismic	
3	269.4	234.3	10.21	210.1	182.7	15.79	188.6	164.0	20.10
4	238.9	207.7	12.45	174.7	151.9	17.75	165.3	143.7	21.75
5	215.5	187.4	14.29	166.4	144.7	19.55	150.0	130.4	22.75
6	175.9	152.9	15.43	148.8	129.4	20.41	131.8	114.6	22.78
7	168.2	146.2	16.71	136.3	118.5	20.98	125.6	109.3	23.23
8	144.4	125.5	17.28	126.9	110.3	21.36	121.0	105.2	23.56

For SI: 1 inch =25.4 mm, 1 foot =305 mm, 1 ksi = 1 MPa, 1 lb/ft = 14.59 N/m, 1 kip/in = 1751 N/m

#### Notes:

- Capacities are calculated in accordance with AISI S310-16, North American Standard for the Design of Profiled Steel Diaphragm Panels.
- Structural Fastener to Supports: #12-14, 12 inches on center spacing across interior supports, 6 inches on center spacing across end supports.
- Fastener spacing at edge panels parallel to deck ribs: #12-14 spaced as indicated in table.
- Stitch Fastener: #1/4-14 Laptek spaced as indicated in table.
- Structural Fasteners shall be located at the panel edge at each support.
- Minimum 0.059-inch-Thick Steel Supports.
- Panels are not subject to uplift.
- Insulation under the panel is outside the scope of this table.
- Safety Factors for ASD are in accordance with AISI S310-16 Table B1.1 for wind and seismic.



# EVALUATION REPORT

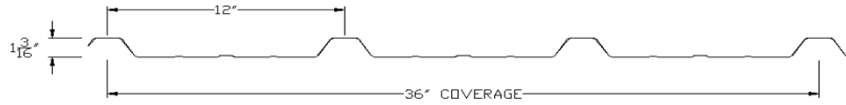
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TABLE 31 – ALLOWABLE (ASD) DIAPHRAGM STRENGTH AND STIFFNESS FOR MULTI-RIB PB, No. 24 GAGE, Fy =50 ksi



Stitch Screw & Edge Fastener Spacing = 12" o.c.									
1-Span			2-Span			3-Span			
Span (ft)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)
	Wind	Seismic		Wind	Seismic		Wind	Seismic	
3	445.3	387.2	18.36	362.5	315.2	26.83	330.6	287.5	32.71
4	395.9	344.3	21.75	326.7	284.1	29.79	301.2	261.9	34.79
5	362.8	315.5	24.34	304.0	264.3	31.74	282.8	246.0	36.02
6	339.3	295.0	26.35	288.3	250.7	33.10	270.3	235.0	36.79
7	321.7	279.7	27.94	276.8	240.7	34.09	261.1	227.1	37.32
8	308.1	267.9	29.22	268.1	233.1	34.83	254.2	221.1	37.69

Stitch Screw & Edge Fastener Spacing = 16" o.c.									
1-Span			2-Span			3-Span			
Span (ft)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)
	Wind	Seismic		Wind	Seismic		Wind	Seismic	
3	403.8	351.1	18.05	310.9	270.3	25.64	275.6	239.7	30.50
4	360.6	313.6	21.31	285.7	248.4	28.42	258.3	224.6	32.54
5	300.3	261.1	23.13	252.4	219.5	29.48	235.6	204.9	33.03
6	281.4	244.7	24.99	244.5	212.6	30.84	220.1	191.4	33.18
7	267.8	232.8	26.49	223.3	194.2	31.03	208.5	181.3	33.18
8	257.6	224.0	27.71	220.2	191.5	31.89	207.8	180.7	33.84

Stitch Screw & Edge Fastener Spacing = 20" o.c.									
1-Span			2-Span			3-Span			
Span (ft)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)
	Wind	Seismic		Wind	Seismic		Wind	Seismic	
3	357.0	310.4	17.64	283.0	246.1	24.90	256.1	222.7	29.59
4	322.3	280.3	20.75	241.7	210.2	26.68	228.1	198.4	30.72
5	300.3	261.1	23.13	233.1	202.7	28.56	210.7	183.2	31.22
6	246.2	214.1	24.12	209.8	182.4	28.99	185.9	161.7	30.50
7	237.6	206.6	25.59	193.1	167.9	29.14	178.3	155.0	30.64
8	204.8	178.1	25.75	180.6	157.1	29.14	172.6	150.0	30.72

For SI: 1 inch =25.4 mm, 1 foot =305 mm, 1 ksi = 1 MPa, 1 lb/ft = 14.59 N/m, 1 kip/in = 1751 N/m

Notes:

1. Capacities are calculated in accordance with AISI S310-16, North American Standard for the Design of Profiled Steel Diaphragm Panels.
2. Structural Fastener to Supports: #12-14, 12 inches on center spacing across interior supports, 6 inches on center spacing across end supports.
3. Fastener spacing at edge panels parallel to deck ribs: #12-14 spaced as indicated in table.
4. Stitch Fastener: #1/4-14 Laptek spaced as indicated in table.
5. Structural Fasteners shall be located at the panel edge at each support.
6. Minimum 0.059-inch-Thick Steel Supports.
7. Panels are not subject to uplift.
8. Insulation under the panel is outside the scope of this table.
9. Safety Factors for ASD are in accordance with AISI S310-16 Table B1.1 for wind and seismic.



# EVALUATION REPORT

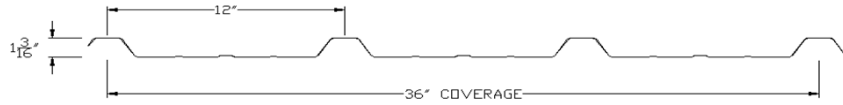
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TABLE 32 – ALLOWABLE (ASD) DIAPHRAGM STRENGTH AND STIFFNESS FOR MULTI-RIB PB, No. 26 GAGE, Fy =80 ksi



Stitch Screw & Edge Fastener Spacing = 12" o.c.									
1-Span			2-Span			3-Span			
Span (ft)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)
	Wind	Seismic		Wind	Seismic		Wind	Seismic	
3	343.9	299.1	11.15	275.9	239.9	17.58	250.0	217.4	22.74
4	303.0	263.5	13.62	246.5	214.4	20.26	225.9	196.4	25.06
5	275.8	239.8	15.66	227.9	198.2	22.21	210.8	183.3	26.60
6	256.4	223.0	17.34	215.1	187.1	23.68	200.6	174.4	27.68
7	239.6	208.3	18.76	205.8	178.9	24.82	193.1	167.9	28.47
8	225.7	196.2	19.96	198.1	172.2	25.73	187.5	163.0	29.08

Stitch Screw & Edge Fastener Spacing = 16" o.c.									
1-Span			2-Span			3-Span			
Span (ft)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)
	Wind	Seismic		Wind	Seismic		Wind	Seismic	
3	312.8	272.0	11.02	237.8	206.7	17.00	209.5	182.2	21.52
4	276.7	240.6	13.43	216.3	188.1	19.54	194.3	169.0	23.73
5	223.9	194.7	15.08	188.0	163.5	20.94	176.0	153.1	24.74
6	207.9	180.8	16.67	179.9	156.4	22.36	162.2	141.0	25.34
7	196.5	170.9	18.01	163.4	142.0	22.97	152.3	132.4	25.71
8	188.0	163.4	19.15	160.4	139.4	23.90	151.2	131.4	26.46

Stitch Screw & Edge Fastener Spacing = 20" o.c.									
1-Span			2-Span			3-Span			
Span (ft)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)	Strength (lb/ft)		Stiffness (kip/in)
	Wind	Seismic		Wind	Seismic		Wind	Seismic	
3	278.3	242.0	10.85	217.4	189.1	16.62	195.3	169.8	21.01
4	247.8	215.5	13.17	181.3	157.6	18.59	171.7	149.3	22.63
5	223.9	194.7	15.08	172.9	150.4	20.41	155.9	135.6	23.58
6	182.8	158.9	16.23	154.8	134.6	21.25	137.1	119.2	23.55
7	175.0	152.2	17.54	141.8	123.3	21.79	130.8	113.7	23.98
8	150.3	130.7	18.08	132.1	114.9	22.13	126.0	109.6	24.28

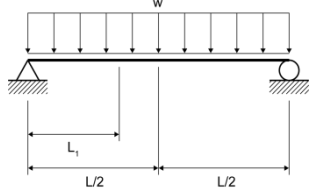
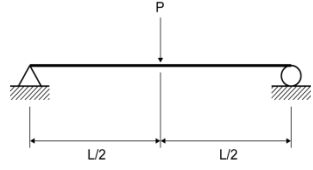
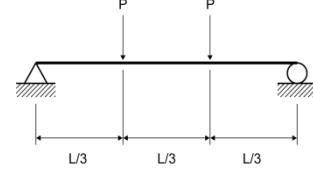
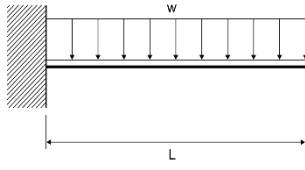
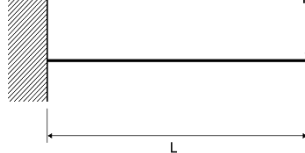
For SI: 1 inch =25.4 mm, 1 foot =305 mm, 1 ksi = 1 MPa, 1 lb/ft = 14.59 N/m, 1 kip/in = 1751 N/m

Notes:

1. Capacities are calculated in accordance with AISI S310-16, North American Standard for the Design of Profiled Steel Diaphragm Panels.
2. Structural Fastener to Supports: #12-14, 12 inches on center spacing across interior supports, 6 inches on center spacing across end supports.
3. Fastener spacing at edge panels parallel to deck ribs: #12-14 spaced as indicated in table.
4. Stitch Fastener: #1/4-14 Laptek spaced as indicated in table.
5. Structural Fasteners shall be located at the panel edge at each support.
6. Minimum 0.059-inch-Thick Steel Supports.
7. Panels are not subject to uplift.
8. Insulation under the panel is outside the scope of this table.
9. Safety Factors for ASD are in accordance with AISI S310-16 Table B1.1 for wind and seismic.



**TABLE 33 – DIAPHRAGM SHEAR WEB DEFLECTION EQUATIONS**

Type of Loading	Loading Condition	Shear Deflection	
Simple Beam at Center	Uniform Load, w	$\Delta_w = \frac{wL^2}{8bG'}$	
Simple Beam at L <sub>1</sub>	Uniform Load, w	$\Delta_w = \frac{q_{ave}L_1}{G'}$	
Simple Beam at center	Point Load, P	$\Delta_w = \frac{PL}{4bG'}$	
Simple Beam at 1/3 points	Point Loads, P	$\Delta_w = \frac{PL}{3bG'}$	
Cantilever Beam at End	Uniform Load, w	$\Delta_w = \frac{WL^2}{2bG'}$	
Cantilever Beam at End	Point Load, P	$\Delta_w = \frac{PL}{bG'}$	

Where:

- b = Depth of diaphragm (ft)
- G' = Stiffness factor (kips/in)
- L = Diaphragm Length (ft)
- L<sub>1</sub> = Distance to point where deflection is calculated (ft)
- P = Concentrated load (lbs)
- q<sub>ave</sub> = Average diaphragm shear (lbs/ft)
- w = Uniform load (lbs/ft)
- Δ<sub>w</sub> = Web deflection (in.)